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A Review of Empirical Methods for Runoff Estimation and Their GIS Applications

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Abstract: ***Irregularities in rainfall patterns, frequent floods, and droughts in different parts of the world result from the effects of global climate change. In such dynamic climatic conditions, sustainable management of water resources is a challenging task under a river basin's complex process response system. In this perspective, hydrological studies are being carried out to estimate the runoff water yield and understand physico-climatic controls require further development in methodologies. Research criteria have advanced and empirical method's development and applications have advanced significantly. Empirical methods quantify the runoff volume by correlating the amount of rainfall with the number of influencing basin factors. In recent times, empirical runoff estimation has become popular because of its simplicity and correlation assessment capacity with multiple associate factors. This study aims to have an overview of the empirical runoff estimator SCS-CN method and other empirical equations. This study has covered the primary areas in hydrological analysis are runoff estimation, groundwater recharge, and runoff-soil erosion estimation using empirical methods. The authors have suggested the advantages, disadvantages, and possible areas of development of the widely used SCS-CN method and other empirical methods of runoff estimation.***
Keywords: SCS-CN, Empirical equations, Runoff estimation, GIS, RUSLE

I. INTRODUCTION

The overland flow of excess rainfall is called surface runoff and initially joins a stream network in a basin system. Through a series of channel networks, the runoff water eventually enters the master stream. The volume of water measured at the basin outlet is known as stream flow or total runoff, and it consists of base flow, interflow, and overland flow (Betson, 1964). The total volume of surface runoff is influenced by several factors, such as the rate of infiltration, soil properties, rainfall intensity, vegetation cover, basin morphometry, and meteorological conditions (Kothyari & Garde, 1991). A major component of hydrological modelling is the total runoff or streamflow. Therefore, measurement of runoff is a crucial part of any hydrological modelling (S. Verma et al., 2017). In recent decades, the SCS-CN method has been widely applied in different parts of the world to estimate runoff depth and volume (Abdi & Meddi, 2021; Rajasekhar et al., 2020). SCS-CN method was developed by the United States Department of Agriculture (1956) and is a widely used simple, reliable, and attractive method for estimating runoff from given rainfall (Sangin et al., 2023). This approach necessitates the use of easily available tables and curves as well as a simple empirical formula. SCS-CN method uses minimal numbers of input parameters. Here, CN denotes the runoff potentials that are measured using LULC, different types of antecedent moisture conditions, and soil types (Kadam et al., 2012; Rawat & Singh, 2017).

The framework of this review study is categorized under two broad classes (Fig. 1) as Model-based runoff estimation and application: SCS-CN, and Runoff Estimation and application using equations.

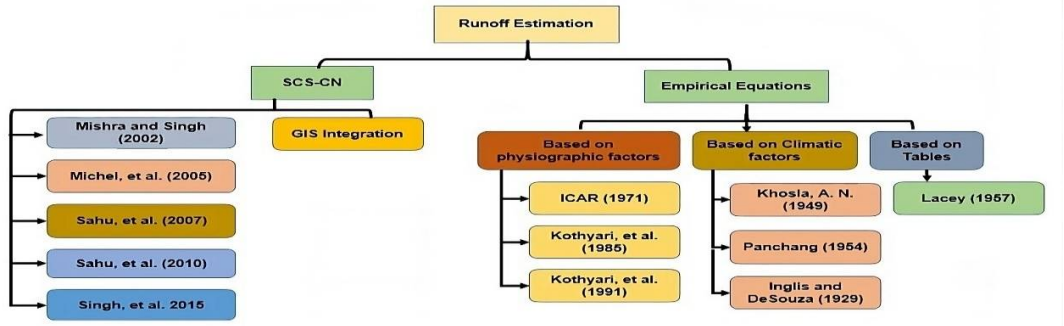


Fig. 1 Review Framework

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A. Modifications of SCS-CN method

Initially, this method was designed for use in small catchments. SCS-CN method has been modified several times for its wider application in varied watersheds. Usually, the original SCS-CN is a relationship between rainfall and runoff volume (Michel et al., 2005; Sahu et al., 2010, 2012; R. K. Verma et al., 2021). SCS-CN method consists of the following equations (Eq. 1-3):

$$Q = \begin{cases} \dfrac{P - I_a{}^2}{P - I_a + S}, \text{ if } P > I_a \end{cases} \quad (1)$$

and $I_a = l\,S$ (2)

where, Q = direct runoff (mm), P = total precipitation (mm), I_a = initial abstraction (mm), S = potential maximum retention (mm), and l = initial abstraction coefficient. The parameter S is expressed as follows (Eq. 3):

$$S = \frac{25400}{CN} - 254 \quad (3)$$

where CN denotes the Curve Number and CN depends on soil type, LULC, surface condition, and antecedent moisture conditions.

The basic proportionality hypothesis of the SCS-CN approach was approximated by Mishra and Singh (2002) as runoff coefficient (C) = degree of saturation (Sr), where $Sr = \left(F / S\right)$ and $C = \left(Q / P - I_a\right)$ (Verma et al., 2021). They altered the direct surface runoff by adding antecedent moisture (M) (Eq. 4-5) as –

$$Q = \frac{\left(P - \lambda s\right)\left(P - \lambda S + M\right)}{\left(P - \lambda S + M + S\right)} \quad (4)$$

$$M = 0.5(-1.2S + \sqrt{0.6S^2 + 4P_5S} \quad (5)$$

Mitchel et al. (2005) modified the original SCS-CN to estimate the runoff at any instant during a storm. When soil moisture level is high, major rainfall volume is converted to runoff and the total rainfall will directly be converted to runoff if the soil moisture level is saturated. They proposed the following equation (Eq. 6) accounting for soil moisture:

$$V = V_0 + P_t - Q_t \quad (6)$$

where, V_0 = soil moisture level at the starting of rainfall (mm), P_t = accumulated rainfall at time t during a storm, and Q_t = accumulated runoff at time t during a storm (mm), and V = soil moisture level (mm).

Mitchel et al. (2005), addressed several inconsistencies in the original method. They mentioned confusion raised between intrinsic parameters and initial condition, and also from the incorrect use of the underlying soil moisture accounting procedure (Sahu et al., 2010). They clarify $\left(V_0 + I_a\right)$ as an intrinsic parameter (S_a) of the soil moisture model. MITCHEL (Mitchel et al., 2005) model computes the following equations (Eq.7-9) for runoff estimation:

$$\text{If } V_0 \le S_a, \text{ then } Q = 0 \quad (7)$$

$$\text{If } S_a - P < V_0 < S_a, \text{ then } Q = \frac{\left(P + V_0 - S_a\right)^2}{P + V_0 - S_a + S} \quad (8)$$

$$\text{If } S_a \le V_0 \le S_a + S, \text{ then } Q = P\left[\frac{S + S_a - V_0{}^2}{S^2 + S + S_a - V_0\ P}\right] \quad (9)$$

For instance, Sahu et al. (2007) suggested an improvement for V_0 of the MICHEL model. They implemented two assumptions to overcome the gap of Michel et al. (2005) method, such as – (i) 5 days before the onset of rainfall the older moisture level is zero or a fraction of S, and (ii) the initial soil moisture level (V_0) is equal to the sum of previous moisture level (V_{00}) at the beginning of rainfall and a small amount of that rainfall is not converted to runoff because of 5 days antecedent rainfall. Sahu et al. (2007) expressed these two assumptions (Eq. 10-11) as follows:

$$V_{00} = \gamma S, \text{ where } \gamma = 0.0 \text{ to } 0.1 \quad (10)$$

$$V_0 = V_{00} + b(P_5 - Q_5) \quad (11)$$

where, P_5 = 5 days antecedent rainfall, Q_5 = runoff corresponding to P_5

Sahu et al. (2007) considered Eq. 7-9 from Michel et al. (2005) as valid for $P = P_5$. Using Eq. 11 Sahu et al. have suggested the following equations (Eq. 12-14) to obtain runoff for different conditions:

If $V_{00} \leq S_a - P_5$, then $Q_5 = 0$ combined with Eq. 11 gives –

$$V_0 = V_{00} + b P_5 \quad (12)$$

Similarly, if $S_a - P_5 < V_{00} < S_a$, then

$$V_0 = V_{00} + \beta\left[P_5 - \frac{P_5 + V_{00} - S_a}{P_5 + V_{00} - S_a + S}\right] \quad (13)$$

and if $S_a \leq V_{00} \leq S_a + S$, then

$$V_0 = V_{00} + \beta P_5\left[\frac{S + S_a - V_{00}}{S^2 + S + S_a - V_{00}\ P_5}\right] \quad (14)$$

Eq. 10 and Eq. 12 to Eq.14 are referred to as the SAHU model. Sahu et al. (2010) simplified the previous model named Sahu et al. (2007) with $a = 0.1$, $b = 0.4$ and $V_{00} = 0$. This simplified 1-parameter model is further referred to as the SIMP-SAHU model or Sahu et al. (2010). The following expressions (Eq. 15-19) are suggested by Sahu et al. (2010):

$$\text{If } P_5 \leq 0.1S \text{ then, } V_0 = 0.4P_5 \quad (15)$$

$$\text{If } P_0 > 0.1S \text{ then, } V_0 = S\left[\frac{0.44P_5 - 0.004S}{P_5 + 0.9S}\right] \quad (16)$$

From known V_0, Q can be computed as –

$$\text{If } V_0 + P \leq 0.1S, \text{ then } Q = 0 \quad (17)$$

$$\text{If } 0.1S < V_0 + P < 0.1S + P, \text{ then } Q = \frac{(P + V_0 - 0.1S)^2}{P + V_0 + 0.9S} \quad (18)$$

$$\text{If } 0.1S \leq V_0 \leq 1.1S, \text{ then } Q = P\left[1 - \frac{1.1S - V_0}{S^2 + 1.1S - V_0\ P}\right] \quad (19)$$

Mishra and Singh's (2002) approach used by Singh et al. (2015) to suggest an enhanced SMA technique. The process includes the absolute potential maximum retention (S_b), threshold soil moisture (S_a), and initial moisture (V_0). Eq. 23 is used to determine the initial soil moisture, or V_0, while Eq. 24 and 24 are used to determine S_a and S_b (Abdi and Meddi, 2020). Eq. 21–22 are used to calculate the runoff.

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$$R = 0 \text{ If } V_0 \le S_a - P \quad (20)$$

$$R = \frac{P + V_0 \quad P + V_0 - S_a}{P + S + V_0} \text{ if } S_a - P \le V_0 \le S_a \quad (21)$$

$$R = P\left(1 - \frac{S_b - V_0}{SS_b + P \quad S_b - V_0}^2\right) \text{ if } S_a \le V_0 \le S_b \quad (22)$$

$$V_0 = \alpha\sqrt{P_5 \times S} \quad (23)$$

$$S_a = \beta \times S \quad (24)$$

$$S_b = S_a + S \quad (25)$$

Golian et al. (2010) simulated spatial rainfall patterns using the Monte Carlo simulation technique in the HEC-HMS model. They adopted SCS-CN and Green-Ampt and Muskingum infiltration methods as hydrologic models and presented the best result with Green-Ampt and Muskingum compared to the SCS-CN method. In their 2010 study, Bhadra et al. evaluated the effectiveness of SCS-CN and ANN for rainfall-runoff modelling. They reported ANN had the best performance for runoff modelling.

Two categories of empirical studies are frequently used in runoff estimation and hydrological analysis such as – GIS integrated SCS-CN method, and Empirical equations.

B. GIS integrated SCS-CN method

In the last decades, the motivation for applying GIS integrated SCS-CN method to determine the suitable sites for rainwater harvesting, structuring recharge shafts, ponds, etc. has attracted the attention of researchers and hydrologists across the world (Karunanidhi et al., 2020; Rajasekhar et al., 2020; Singhai et al., 2019). This combined application has made significant changes in monitoring and prediction issues in hydrological analysis. GIS-based applications such as MCDM, overlay, spatial analysis, models, etc., are widely used in hydrological modelling (Kadam et al., 2012; Pandey et al., 2021; Pandi et al., 2020) (Fig. 2). Some of the reviewed articles are provided in Table 1.

Fig. 2 GIS-based predictions using SCS-CN model

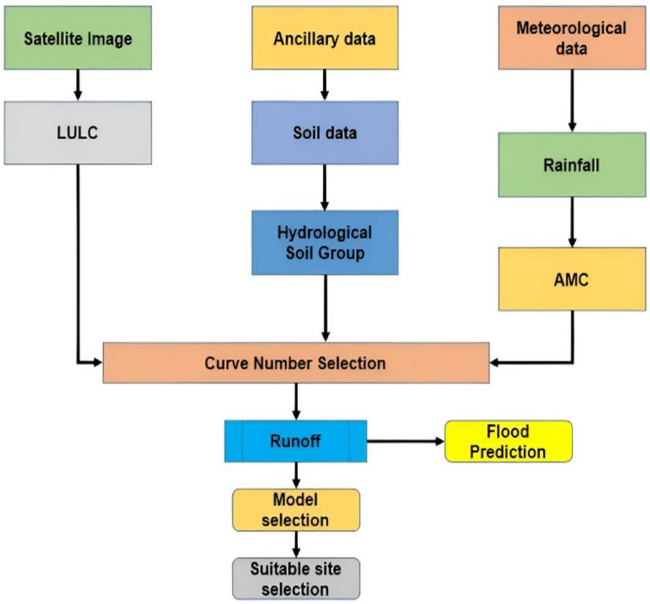


Table 1: Details from the examined literature regarding the SCS-CN method's applicability in GIS

Reference	Study area	Purpose	Method	Suggestion
Darji (2020)	Machhu and Watrak River basins, India	Evaluation of estimated runoff under different geo-environmental conditions	SCS-CN	SCS-CN is applicable in varied environmental conditions
Mohammed et al., (2020)	Manair river basin, India	Modification of SCS-CN based on geo-location of the study area	SCS-WCN	Site suitability selection for artificial groundwater recharge
Gabale & Mangesh Deshpande, (2020)	Sudha watershed, Indrayani sub-basin, India	Estimate surface runoff volume	SCS-CN	Rainwater harvesting, flood mitigation
Karunanidhi et al., (2020)	Lower Bhavani Basin, India	Identify potential surface runoff areas	SCS-CN	Recommended for check dams, percolation ponds, farm ponds, and artificial recharge shafts
Rao (2020)	Gosthani river basin, India	Runoff estimation and evaluation of basin morphometry to understand hydrological processes	SCS-CN	Sufficient runoff records to evaluate the model performances and quantify runoff volume
Rawat & Singh, (2017)	Jhagrabaria watershed, India	Daily runoff calculation and developing the best Empirical Mathematical Model	SCS-CN	Water resources development and management to mitigate hazards and scarcity
Aragaw & Mishra (2022)	Measa, Aposto, Melka Kutire, and Hombole Watersheds of Ethiopia	Clarifies the issues of the SCS-CN method and develops a long-duration SCS-CN-based proxy model	SCS-CN model and SCS-CN-based long-duration model	1-parameter Model 9 is the best alternative.
Shi et al. (2017)	Jiuyuangou, Nanyaogou, and Peijiamaogou watersheds, Suide County, Shanxi Province	Modification of Singh's method and comparison with selected existing models	Original SCS-CN, Singh et al., and proposed SMA-based SCS-CN method.	Reduction of dimensionality of the proposed method to make it simple
Kadam et al., 2012	Upper Karha watershed, India	Suitable site selection for rainwater harvesting structures	Geospatial analysis, Overlay operation, and SCS-CN method	Integration of SCS-CN with geospatial techniques provides better accuracy than ordinary methods

C. Empirical Rainfall-Runoff Relationship: India

Empirical equations are developed for regional applications. These equations have been formulated for runoff estimation from Indian catchments for over a century. Inglis and DeSouza (1929) based on 53 stream gauging sites in Western Ghats and plains developed a method for runoff estimation (Subramanya, 2013) (Eq. 25-26).

For Ghat Regions in Western India (cm)

$$R = 0.85P - 30.5 \quad (26)$$

For Deccan Plateau (cm)

$$R = \frac{1}{254} P\left(P - 17.8\right) \quad (27)$$

where, R = Runoff, P = Rainfall

Khosla, A. N. (1949) first time used temperature as a loss factor in his method to estimate runoff yield over any catchment (Geol & Chander, 2002; Ramasastri & Seth, 1985; Subramanya, 2013) (Eq. 28-29).

For the monthly period

$$R_m = P_m - L_m \quad (28)$$

For L_m ,

$$L_m = 0.48T_m \text{, where } T_m > 4.5^\circ C \quad (29)$$

For the annual period

$$R = P - \frac{T}{3.74} \quad (30)$$

where, R and R_m = Annual and monthly runoff (cm), P and P_m = Annual and monthly precipitation (cm), L_m = Monthly losses (cm), T and T_m =Annual and monthly temperature (°C).

Panchang (1954) modified the Khosla's method by considering the evaporation and transpiration as a loss factor. He mentioned that evaporation losses increase as temperature increases (Ramasastri & Seth, 1985). Thus, he modified the equation (Eq. 31) as –

$$R_A = \sum R_m = \sum P_m \frac{e^{-(T_m - 32)}}{\alpha} \quad (31)$$

where, R_A = Annual runoff, R_m = monthly runoff, P_m = monthly rainfall, T_m = monthly precipitation, α = basin constant, e = loss factor.

Based on Barlow's table, Lacey (1957) developed an equation for Indo-Gangetic plains (Geol & Chander, 2002). He expressed the formula (Eq. 32) as –

$$R = \frac{P}{1 + \frac{304.8}{P}\left[\frac{F}{S}\right]} \quad (32)$$

where, R = Runoff, P = Rainfall, F = Monsoon duration factor, S = catchment factor

Indian Council of Agricultural Research (ICAR, 1971) formulated an equation (Eq. 33) based on 17 sub-watersheds in the Nilgiri Hills (Geol & Chander, 2002). In this method, they used physiographic parameters of the sub-basins. ICAR suggested the following expression –

$$R = \frac{P^{1.44} \times A^{0.63} \times \Delta H^{0.66}}{15.19 \times F_f^{2.05} \times L_a^{2.05} \times T^{1.34}} \quad (33)$$

where, R = Runoff, ΔH = maximum height difference of Watershed, F_f = Factor of Watershed Shape, L_a =Length of the main waterway in km, P = Rainfall, A = Watershed Area, T = Temperature

Kothyari et al. (1985) formulated an empirical equation (Eq. 34-35) to estimate the runoff yield based on 26 river catchments across India (Kothyari & Garde, 1991). For the first time, they considered the vegetation cover factor in their study as a runoff influencing factor. They used four types of land cover such as forest cover, grass, arable land, and wasteland, and combinedly termed them as vegetation cover.

$$F_v = \frac{\left(a_1 F_F + a_2 F_G + a_3 F_A + a_4 F_W\right)}{A} \times 100 \quad (34)$$

$$R_m = \frac{F_v^{0.49}(P_m - 0.5T_m)^{1.59}}{26.5} \quad (35)$$

where, R_m = Monthly Runoff , F_v = Vegetal cover factor, P_m = Monthly Rainfall, T_m = Monthly Temperature, F_F = Percentage of forest cover, F_G = Percentage of grass and scrubland, F_A = Percentage of arable land, F_W = Percentage of wasteland, $a_i = (1-4)$: Weight factor, A = Watershed area

Kothyari et al. (1991) suggested another equation due to poor performance in the previous equation. They used the data from 55 catchments across India. Kothyari & Garde (1991) compared the two methods and presented a better performance of the modified method (Eq. 36)

$$R_m = C(P_m - 1.4T_m)^{0.90} F_v^{0.20} A^{0.04} T_m^{-0.95} \quad (36)$$

where, C = basin constant, R_m = Monthly Runoff, F_v = Vegetal cover factor, P_m = Monthly Rainfall, T_m = Monthly Temperature

D. Runoff estimation equations used in RUSLE model

The RUSLE model is a widely used GIS-integrated empirical soil erosion estimation model. RUSLE model modified from parent model USLE (Chakrabortty et al., 2020; Djoukbala et al., 2018; Prasannakumar et al., 2011; Singh et al., 2023). This model requires six parameters to estimate long-term annual soil erosion, these are - (i) Rainfall erosivity factor (R), (ii) Soil erodibility factor (K), (iii) Slope length factor (L), (iv) Slope steepness factor (S), (v) Plant Cover and management practice factor (C), and (vi) Conservation service factor (P). Multiplying these six factors annual soil erosion is estimated (Belayneh et al., 2019). Rainfall erosivity factor requires a runoff estimation equation and researchers worldwide mostly use empirical equations. This method is not homogenous to the researchers.

Arekhi et al. (2012) for their study of the Ilam dam watershed, in Iran, used the R-F relationship (Eq. 37-39) developed by Renard and Freimund (1994).

$$R - \text{factor} = 0.07397 \times F^{1.847} \quad / 17.2 \text{ when } F < 55 \text{ mm} \quad (37)$$

$$R - \text{factor} = 95.77 - 0.681 \times F + 0.477 \times F^2 \quad / 17.2 \text{ when } F \geq 55 \text{ mm} \quad (38)$$

$$F = \frac{\sum_{i=1}^{12} P_i^2}{\sum_{i=1}^{12} P} \quad (39)$$

where, F = Fournier index, P_i = mean rainfall depth in mm for month $_i$, P = mean annual rainfall in mm

In India, Rajbanshi and Bhattacharya (2020) used the equation (Eq. 40) which was developed by Singh et al. (1981) for the Konar basin, India.

$$R = 79 + 0.363 \times Xa \quad (40)$$

where, R = annual erosivity index, Xa = average annual rainfall (mm)

Chakraborty et al. (2020) utilized an equation (Eq. 41) developed by Arnoldus (1980) for the Arkosa watershed of the Dwarakeswar River basin, West Bengal, India.

$$R = \sum_{i=0}^{12} 1.735 \times 10^{\left(1.5\log_{10}\left(\frac{P_i^2}{P}\right) - 0.08188\right)} \quad (41)$$

Where, R = rainfall erosivity factor, P_i = monthly rainfall (mm), P = annual rainfall (mm)

Ghosal and Bhattacharya (2020) in their study categorized the runoff estimation equations according to their different climatic zones (Table 2). They have identified seven climatic zones across the world. From their study, it is clear that researchers preferred to apply selective equations for a specific geo-location.

Table 2: Reviewed information about runoff estimation equations (Eq. 42-52) for the RUSLE model (Ghosal & Das Bhattacharya, 2020)

Reference	Equation	Climatic zone
Loureiro and Coutinho (2001)	$R = \frac{1}{N}\sum_{i=1}^{12}\left(\sum_{m=1}^{12} 7.05\text{rain}_{10} - 88.92\ \text{days}_{10}\ \ m,i\right)$ (42) where, N = no. of observations, rain_{10} = when monthly rainfall ³ 10 mm otherwise the value is zero, days_{10} = no. of days in a month when rainfall ³ 10 mm, m i	Tropical wet climate
Roose (1996)	$R_{10} = r \times a$ (43) where, R_{10} =runoff erosivity factor, r = mean annual rainfall, a = constant	
Kassam (1980)	$R = 117.6 \times\ 1.00105^{\ AAP}$ (44) where, ^{AAP} = average annual rainfall	
Hurni (1985)	$R = -8.12 + \ 0.562 \times P$ (45) P = average annual rainfall	Tropical wet and dry climate
Singh (1981)	$R = 79 + 0.363AAP$ (46) where, AAP = Annual average rainfall	
Bu et al. (2003)	$R_j = \ 0.1281 \times I_{30B} \times P_f \ - \ 0.1575 \times I_{30B}$ (47) where, P_f = Annual rainfall (mm), I_{30B} = storm intensity for maximum 30 min (mm/h)	Semi-arid or Steppe Climate
Renard and Freimund (1994)	$R = 587.8 - 1.219 + 0.004105P^2$ (48) where, P = annual rainfall (mm)	Humid Subtropical Climate
Babu et al. (2004)	$R = 81.5 + 0.375 \times r$ (49) where, 340 £ r £ 3500mm	
Kumar and Kushwaha (2013)	$Y_1 = 79 + 0.383X_1 (r = 0.83)$ (50) $Y_2 = 50 + 0.389X_2 (r = 0.88)$ (51) where, X_1 and X_2 = average annual and seasonal rainfall (mm)	Alpine Climate
Sorrentino (2001)	$R = \left(1163.45 + 4.9H - 35.2NGP - 0.58q\right)/100$ (52) where, NGP = average per year rainy day, q = elevation, H = average annual precipitation (mm)	Mediterranean Climate
Irvem et al. (2007)	$R = 0.1215F^{2.2421}$ (53) where, F = Fournier index (Eq. 39), P_i = mean rainfall depth in mm for month$_i$, P = mean annual rainfall in mm	Arid Climate

II. MODEL-BASED RUNOFF ESTIMATION AND APPLICATION: SCS-CN

A. Application of SCS-CN in Runoff depth and volume estimation

The basic application of the SCS-CN method is the estimation of runoff depth and volume. The original SCS-CN method has been modified several times. Sahu et al. (2012) in their study performed a comparative study to check the performances of the modified models outside of USA watersheds. They adopted the Original SCS-CN, Mishra and Singh model, Michel model, Sahu 3p model, Sahu 2p model, and Sahu SIMP model for Kalu and Amba watersheds in India. The selected models performed very well but the Sahu 3p model and Sahu SIMP model performed with higher efficiency comparatively to the others.

Using GIS and remote sensing thematic data layers are generated. Integration of GIS and RS with SCS-CN very accurately estimates the depth and volume of runoff as reported in different studies (Kumar et al., 2021; Saha et al., 2022; Satheeshkumar et al., 2017). Saha et al. (2022) used annual rainfall data for their study. They reported the correlation between rainfall-runoff is 0.98. As they mentioned, the average runoff depth that occurred annually during the study period was 979.45 mm in response to 1341.59 mm of rainfall. The depth of runoff is quite high and 73% of the average annual rainfall. Satheeskumar et al. (2017) used 14 years of rainfall data to estimate the annual runoff depth and volume. They presented that the correlation between rainfall and runoff is 0.84, which means the relation is highly correlated. They measured the average annual volume and depth of runoff are 32682501 m^2 and 181.731 mm where the rainfall was 848.56 mm. The drainage basin of this study drains only 21% of water.

Jiao et al. (2015), in their study, modified the SCS-CN method and used it for runoff prediction for different land practices. They have compared the performance between the original and improved methods and noticed the prediction accuracy varies for each land practice. Their improved SCS-CN method predicted better accuracy than the original one. Singhai et al. (2019) estimated the surface runoff for the period 1997 to 2006. Their estimated value is under-predicted compared to the actual value for each year. The drainage basin yields 42% water from the average rainfall and is available for utilization and management. Kadam et al. (2012) also conducted a similar study to predict the runoff for different land classes using the Arc-CN Runoff tool. To estimate the runoff depth geospatial technology has been used for different watersheds across India (Rawat and Singh, 2017; Rao, 2020; Karunanidhi et al. (2020); Gabale and Deshpande, 2020; Mohammed et al., 2020). They adopted the original SCS-CN method for their study and reported that the SCS-CN method had the best performance for runoff estimation in different physical settings.

B. Suitable Site Selection for Rainwater Harvesting: Application of GIS-based SCS-CN

The construction of rainwater harvesting structures is a management strategy in order to remediate the vulnerabilities of groundwater resources. Percolation tanks, Farm ponds, check dams, and gully plugs are the important rainwater harvesting structures mentioned in different studies worldwide. Kadam et al. (2012) using GIS and remote with SCS-CN method have identified suitable sites for structuring rainwater harvest. They utilized overlay operation in the GIS environment for this purpose and secured 86. 25% average accuracy. The multi-criteria Decision-Making approach was used by Singhai et al. (2017) to define suitable sites for check dams, farm ponds, and percolation tanks. They presented 70-100% overall accuracy in the validation phase. Rajasekhara, M.et al. (2020) adopted the AHP method in order to determine the sites for recharging groundwater. In this study, they considered check dams, farm ponds, and percolation tanks as recharge mediator. They have identified a total of 35 potential sites for harvesting rainwater with 82% accuracy. Multi-influencing factor (MIF) is a geospatial technique used for considering the effects of parameters and their interaction. Pandey et al. (2021) determined the potential recharge zones using the MIF method. For this purpose, they used LULC, soil, slope, drainage density, geomorphology, and depth of water level as influencing factors.

C. Flood Potentiality Estimation: Application of GIS-based SCS-CN

Recently, many researchers have explored the ability of the SCS-CN method along with the GIS interface in the flood potentiality estimation process. Sharma and Singh (2014) used the SCS-CN method to estimate the flood potentialities for different sub-basins within the main study area using parameters like DEM, rainfall, LULC, and soil data. At first, they estimated the runoff potentialities for each sub-watershed, correlated that with gauge data, and found higher accuracy. Abdi and Meddi (2020) emphasized the use of GIS integrated SCS-CN method to determine the flood potentialities in an ungauged river basin. Four models such as Mishra et al. (2006), Sahu et al. (2010), Singh et al. (2015), and Verma et al. (2017) are compared with the original SCS-CN method. They presented the predominance of the modified models over the original SCS-CN model and suggested applying these models in different watersheds for validation purposes.

III. RUNOFF ESTIMATION AND APPLICATION USING EQUATIONS

A. Runoff estimation using Empirical equations

Inglis and DeSouza (1929) developed an equation to estimate annual runoff for the catchments in the Western Ghats and Plains of Western India. They used annual rainfall and coefficients for runoff estimation. The Western Ghats is made up of hard rocks, and a higher runoff amount is expected than the plains. Inconsistency is noticed between the formulas because the equation developed for plains calculates a higher runoff value. Khosla (1949), formulated a runoff estimation equation by addressing the loss factor. He used annual rainfall data, and temperature data to formulate the equation and found it to yield good estimates of annual runoff. Panchang (1954) modified the equation proposed by Khosla (1949). Evaporation increases with the increase in temperature. He introduced the evaporation factor in his method as a loss factor.

Raja Rao and Pentaih (1971) used data from four Andhra Pradesh catchments to investigate the efficacy of Khosla's original and modified equations. They concluded that neither the modified Panchang model nor the estimations from Khosla's original calculation agreed with the runoff values that were observed (Ramasastri and Seth, 1985).
Kothyari et al. (1985) derived a new method for runoff estimation. In this method, he introduced the vegetation factor as a runoff-controlling factor. He used 26 basins to derive the formula. Kothyari et al. (1991) found inconsistencies in their previous method and developed another equation using 55 catchment data across India. They presented the better performance of the modified method compared to the previous method.

B. Soil erosion estimation from runoff

In recent years, soil erosion estimation by using the RUSLE model has been a popular and widely accepted method among researchers. The main parameter of the model is the runoff erosivity (RE) factor. RE factor required an equation to generate the model. There is no single equation that can be utilized across the world. This equation varies significantly among the researchers and from region to region. So, the researchers used regionally developed empirical equations to develop the model. Arnoldus (1980) developed an equation for runoff estimation. It has been noticed that researchers across the world frequently used the equation developed by Arnoldus (1980). It is not possible to address the effectiveness of this method for different watersheds. Because empirical equations are directly applied in models without checking the validity of the equations.

IV. MAJOR FINDINGS

1) Despite several modifications, the SCS-CN method has received a lot of popularity because of its straightforward conceptual approach for runoff prediction in ungauged river basins worldwide.
2) Modifications underpinned the original SCS-CN method in order to achieve region-specific higher efficiency.
3) Integration of SCS-CN with GIS has strengthened the prediction results with an accuracy of more than 80%. The applications of hybrid methods expanded the scope of the study.
4) Considering the basin complexities empirical runoff estimation equations are developed over time to overcome the prediction errors.
5) Empirical runoff estimation equations consider the regional morphometric and climatic parameters that demonstrate improved prediction accuracy.
6) The RUSLE model is simple and suitable for soil erosion predictions. Here, the use of empirical equations exemplifies the desired accuracy because of its regional properties.

V. CONCLUSION AND RECOMMENDATION

Runoff estimation is essential for hydrological modelling, sustainable water resources management, and planning. Runoff is a dynamic process, generated through the complex process-response system of a drainage basin. To estimate the runoff depth for any catchment, there is a need for a method that can correlate the complexities of the basin. In recent times, most of the drainage basins have no proper gauge stations that can monitor the stream runoff depth and other basin parameters across the world. This study attempted to analyze the applications of SCS-CN and empirical equations for runoff estimation based on existing contributions. SCS-CN method presented an outstanding performance with more than 80% accuracy across the globe. Similarly, empirical equations performed well in all formats of runoff estimation. Several scholars utilized empirical equations for runoff and soil erosion estimation. It has been reviewed from the previous studies that all of the methods analyzed in this study performed well with maximum accuracy. This study evaluated the applications of SCS-CN and empirical equations on runoff estimation and allied applications under different physico-climatic conditions. Another purpose of this study was to consider the conventional methods used in different environmental settings for runoff estimation, to uncover the most suitable alternatives for runoff estimation under varied natural processes. Additionally, this review process found that the modified methods under a specific natural system presented higher accuracy than the original methods. Another important outcome of this study is the application of runoff estimation methods along with GIS opened multiple dimensions for hybrid study. This study recommends that researchers use the updated SCS-CN method and empirical equations for their study. This study demonstrates problems related to runoff estimation when considering moisture conditions and other factors. To resolve this issue, future researchers can develop a framework integrating GIS and runoff estimation methods to achieve better model performances. Future researchers can use hybrid models that consider the wide range of physico-climatic controls for estimating runoff depth and allied applications.

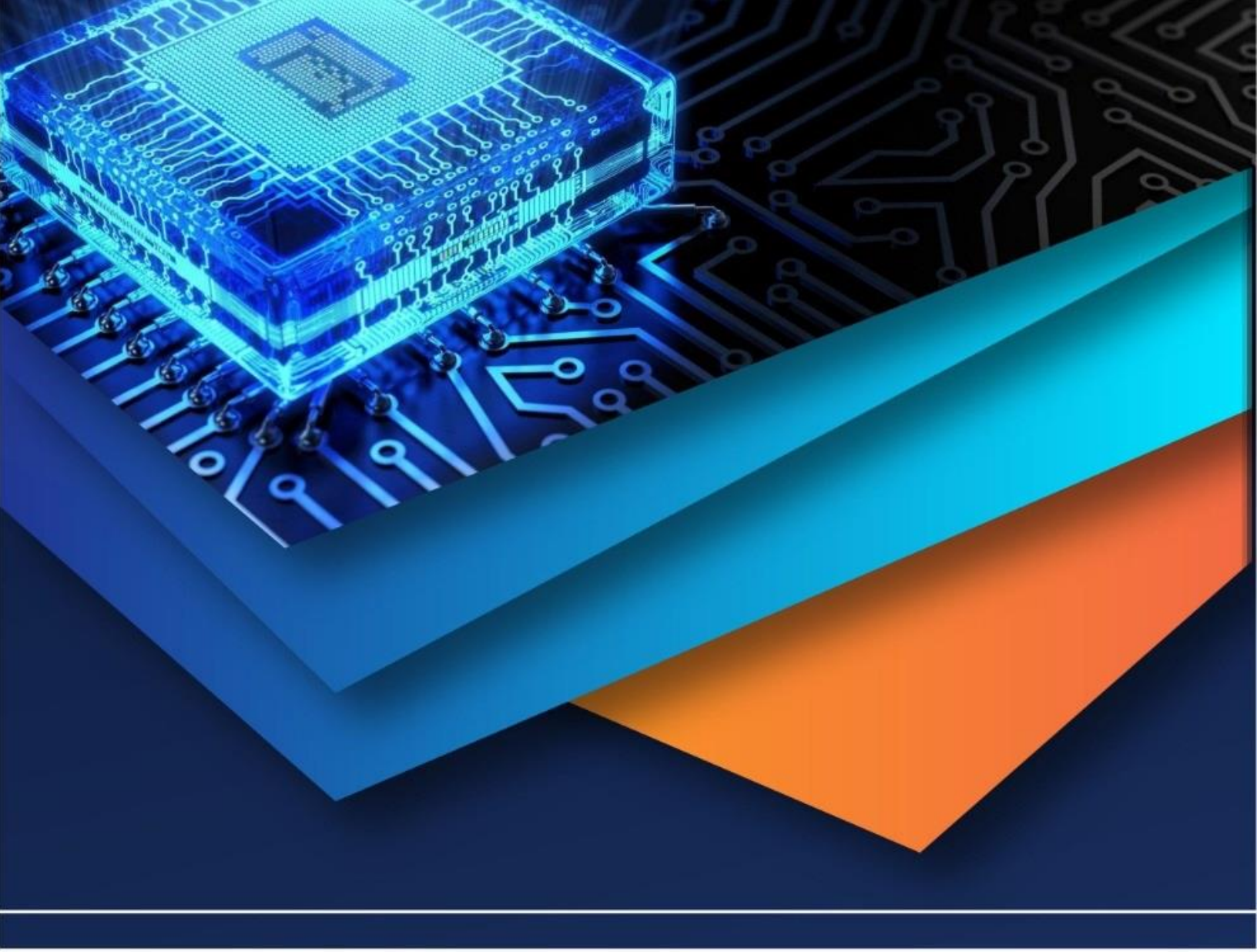
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