



# iJRASET

International Journal For Research in  
Applied Science and Engineering Technology



---

# INTERNATIONAL JOURNAL FOR RESEARCH

IN APPLIED SCIENCE & ENGINEERING TECHNOLOGY

---

**Volume: 14      Issue: I      Month of publication: January 2026**

**DOI:** <https://doi.org/10.22214/ijraset.2026.76775>

**www.ijraset.com**

**Call:**  08813907089

**E-mail ID:** [ijraset@gmail.com](mailto:ijraset@gmail.com)

# A Review: Cost Analysis of RC Buildings in Different Seismic Zones

Yogesh K. Tarare<sup>1</sup>, Mr. Aaquib Ansari<sup>2</sup>

<sup>1</sup>Student, Department of Civil Engineering, G.H. Raisoni College of Engineering & Management, Nagpur, Maharashtra, India

<sup>2</sup>Assistant Professor, Department of Civil Engineering, G.H. Raisoni College of Engineering & Management, Nagpur, Maharashtra, India

**Abstract:** Seismic zone variations across India from [Zone II to V], significantly influence RC building costs through base shear amplification, member reinforcement increases and ductility detailing requirements per IS 1893:2016 with Zone V structures costing 25 to 40% more than Zone II equivalents for identical G+10 configurations. This review synthesizes 20 studies (2015 to 2025) analysing cost implications of seismic coefficients ( $Z=0.10$  to  $0.36$ ), response reduction factors ( $R=3$  to  $5$ ) and importance factors ( $I=1.0$  to  $1.5$ ) across building heights, materials ( $M25$  to  $M40$ ) and analysis methods (ETABS static/RSM). Zone III to IV transitions add 15 to 25% to concrete/rebar volumes; soft storey irregularities amplify costs 20 to 30% through stiffness upgrades. Quantity estimation reveals 12 to 18% steel increase per zone increment, while P-Delta effects in tall buildings add 8 to 12% to lateral systems. Findings establish cost-index relationships ( $\text{₹}/\text{sqm}$  vs  $Z$ -factor) enabling economic Zone V design through optimized  $R$ -factors and regular geometry. Gaps identified include lifecycle costing, hybrid material optimizations and Zone V field validations.

**Keywords:** RC buildings, seismic zones, cost analysis, base shear, IS 1893:2016, ETABS, reinforcement quantity, response reduction factor, ductility detailing, construction economics.

## I. INTRODUCTION

Rapid urbanization across India has accelerated mid-rise RC construction (G+10 typical), where seismic zoning per IS 1893:2016 governs design economy through zone factors  $Z=0.10$  to  $0.36$  that amplify base shear 3.6x from Zone II to V, directly increasing reinforcement 25 to 40%, concrete volumes 10 to 20%, and total costs ₹1,400 to 2,500/sqm. While Zone II permits economical M25/Fe415 designs, Zone V demands M30 to M40 concrete, Fe500 steel, and IS 13920 ductile detailing that elevate construction expenses 30 to 35% for identical Length 20m x Breadth 15m plans, challenging developers in high-risk Himalayan/northeastern regions. Historical failures—Bhuj 2001, Latur 1993—underscore inadequate seismic provisions causing disproportionate collapses despite similar gravity demands, highlighting need for zone-specific cost-performance optimization.

ETABS facilitates equivalent static/response spectrum analysis incorporating  $Z$ ,  $I=1.0$  (residential),  $R=5$  (SMRF) factors across soil Type II, revealing progressive member up-sizing: columns 450x450mm (Zone II) to 600x600mm (Zone V), beams 300x550mm to 350x650mm with 20 to 30% rebar escalation. Support reactions escalate 40 to 65% exterior/edge columns, while interior variations remain <10%, concentrating cost penalties in perimeter systems. IS 456:2000, IS 875 gravity loads remain constant, isolating seismic coefficient as primary cost driver.

CPWD Schedule of Rates 2023 quantifies impacts: Zone V adds ₹800 to 1,100/sqm through steel (₹65/kg), concrete (₹5,500/m<sup>3</sup>), formwork (₹250/m<sup>2</sup>), and labor premiums for ductile hooks/spacing. Literature gaps persist in lifecycle costing, hybrid optimizations (shear walls vs frames), and Zone V field validations despite 25 to 35% premium established empirically. Present review targets G+6 RCC residential (Length 20m x Breadth 15m, 3m height/storey) across Zones II to V using ETABS-derived quantities validated against IS 1200 BOQ, establishing cost-index curves ( $\text{₹}/\text{sqm}$  vs  $Z$ -factor) and optimization strategies minimizing 30% premiums through  $R$ -factor maximization and regularity. Objectives encompass steel/concrete escalation quantification, percentage cost variance II→V, and material-efficient configurations ensuring IS 1893 compliance.

## II. LITERATURE REVIEW

M Nagarajan et al. (2025) proposed probabilistic seismic risk frameworks for RC buildings in crustal/subduction zones, quantifying Zone V cost premiums at 25 to 35% via probabilistic BOQ (steel 65 to 85 kg/sqm). Monte Carlo simulations on G+8 frames indicated  $R=5$  optimizations reduce total costs 12% over IS 1893 equivalents, validating perimeter reinforcement dominance and hybrid shear walls for 10% savings in high-hazard northeastern India.

Mohammed Moizuddin et al. (2025) compared G+20 RCC seismic performance across Zones II to V using ETABS response spectrum, noting base shear 3.5x rise drives 28 to 36% cost escalation in Zone V (concrete 0.14 m<sup>3</sup>/sqm, steel 75 kg/sqm). Longitudinal reinforcements increase 30% in exterior columns, with CPWD SOR validating 2,200 sqm rates; regularity caps premiums at 25% via R=5 SMRF.

Ishaan Trikha et al. (2025) performed comparative seismic analysis of symmetric/asymmetric RCC using ETABS equivalent static method, revealing asymmetry amplifies Zone V costs 22 to 32% through torsional rebar (columns Ast+25%). Symmetric G+8 plans maintain 1,800 to 2,300 sqm via regularity, aligning with IS 1893 for 10 to 15% savings over irregulars in Zones III to V.

G Dong et al. (2024) reviewed optimum seismic designs of RC frames, proposing uniform damage optimization reducing Zone V costs 10 to 18% via adaptive inter-storey drift in ETABS. For mid-rise Indian buildings, solutions minimize steel (Ast 4 to 6%) and lifecycle repairs by 22%, highlighting gaps in IS code conservatism and advocating performance-based hybrids for 15% savings in high-seismic regions.

Satwik P Rayjada, Jayadipta Ghosh, Meera Raghunandan (2023) conducted seismic life-cycle cost analysis of Indian RC buildings accounting for hazard uncertainty, finding Zone III to V premiums 20 to 40% driven by P-Delta and soil-structure effects. Fragility curves for G+10 residential showed M40 upgrades and IS 13920 detailing add 8 to 12% upfront but save 25% in expected losses, recommending TLCC over force-based IS 1893 for economic zoning.

Allavarapu Durga Bharat et al. (2023) analyzed concrete vs. steel RC with shear walls in seismic zones via ETABS, finding hybrid RC cuts Zone V costs 20% over pure frames (rebar 25% less). Static/RSM showed shear walls reduce drifts 35%, lowering total superstructure 15 to 28% via IS 1200 quantities, ideal for G+6 to G+10 Indian residential with Fe500 ductility.

PS Badal et al. (2022) framed probabilistic performance integration in prescriptive RC designs per Indian codes, reducing Zone IV to V vulnerabilities and costs 15 to 20% through drift-based checks. Applied to G+12 frames, it optimizes BOQ (steel down 18%) against IS 1893 overdesign, emphasizing lifecycle economics and shear wall additions for 12% savings in irregular tall structures.

Mehta and Jadhav (2022) optimized Zone V costs for RC buildings using M40 concrete and hybrid frames, achieving 10 to 15% savings over conventional M25/Fe415 designs while maintaining R=5 SMRF ductility per IS 1893:2016. ETABS analysis on G+8 structures reduced column steel by 18% via 600x600mm sections and targeted shear walls, lowering total BOQ 12% despite ductility detailing. Findings highlight material upgrades capping Zone V premiums at 28% vs 36% baseline, with gaps in field validations for northeastern India.

SC Dutta et al. (2021) assessed seismic vulnerability of low to mid-rise RC buildings in Indian zones via fragility analysis and non-linear static methods, revealing Zone IV to V structures incur 20 to 30% higher reinforcement costs due to amplified base shear (Z=0.24 to 0.36). ETABS models showed drift limits demand 15 to 25% steel escalation in SMRF frames, with lifecycle premiums rising 18% from ductility detailing per IS 13920, emphasizing economic retrofits for G+6 to G+10 plans.

Nagamani and Mahalakshmi (2019) designed G+6 RC buildings across Zones II to V using ETABS and BOQ estimation, reporting cost escalation from 1,500 to 2,400 Rs/sqm (60% rise) driven by exterior column steel increases of 35% (16 to 22 nos 20mm bars). Beams required 28 to 46 nos 16mm rebar, confirming reinforcement dominance (65 kg/sqm Zone V) per CPWD SOR 2019. Study validates Z-factor linearity but notes lifecycle costing gaps for P-Delta in mid-rise.

Borkar and Awchat (2019) modeled G+6 RC frames in Zones II to V via ETABS, finding base shear escalation from 285 to 980 kN (3.4x) and exterior reactions up 42% in columns/beams, yielding total steel 28 to 36T (29% rise). CPWD rates produced 1,650 to 2,250 Rs/sqm, with formwork splitting underrepresented; perimeter systems absorbed 60% premiums. Emphasizes R=5 optimizations for 10% savings, gaps include detailed labor for IS 13920 hooks.

PE Mergos et al. (2018, extended 2024 context) developed optimum seismic designs minimizing life-cycle costs (TLCC) in RC frames, achieving 15 to 20% reductions in Zone V through uniform damage distribution and drift adjustments. Applied to 8 to 12 storeys Indian SMRF, the method cuts initial steel 12% (Fe500) while repair costs drop 30% post-MCE, outperforming code-based ETABS designs by prioritizing R-factor tuning and irregularity avoidance.

Shekharsingh and Suryawanshi (2018) tracked G8 RC progression across zones, noting ground floor steel up 42% in Zone V, displacements 8 to 28mm, and shear walls adding 350 Rs/sqm but reducing drifts 35%. ETABS static/RSM showed R=3 to 5 tuning caps premiums at 30% for tall frames, concentrating costs in exterior columns (Ast+25%). Gaps persist for G+12 validations and hybrid lifecycle economics per CPWD.

Kavita Verma and Rabbani (2018) confirmed G+6 external beam steel from 0.53 to 1.22% (130% rise) and internal 0.77 to 1.40% across Zones II to V using STAAD Pro, with no bottom rebar adjustments per IS 456:26.5.1 despite shear amplification. Total rebar dominated 28% of 1,800 to 2,300 Rs/sqm, emphasizing interior focus gaps; regularity saved 15% via minimized stirrups (150mm cc).

Nilendu Chakrabortty and Lamba (2020) analyzed multi-storey RC in Zones II to V using ETABS, reporting Zone V steel at 53 to 84T vs Zone II 45 to 69T (3x base shear), with column Ast reaching 4 to 6.2% gross area. Linear Z-factor impact drove 25 to 35% costs via Fe500 upgrades and drift limits (0.004h). Validated against IS 456:2000, but soil effects (Type II to III) underexplored for foundation BOQ escalations.

Sandeep Reddy and Reddy (2017) documented frame exterior reactions 41.75 to 64% higher and concrete volumes 1.4 to 4.0 m<sup>3</sup> in Zones II to V, establishing perimeter cost concentration (65% of premiums). BOQ details per IS 1200 showed steel 55 to 75 kg/sqm driving 25% escalation, with Fe415 baseline. Gaps include detailed Zone V labor for ductile hoops under CPWD SOR.

Pankaj Agarwal et al. (2016) compared G+10 RC in Zones II to V, finding stiffness variations 2 to 20x, rebar up 2.1x, and costs 25 to 30% (1,800 to 2,350 Rs/sqm) via ETABS. Zone transitions demanded M30/Fe500, column sizing 450 to 600mm; regularity optimizations reduced 12%. Gaps in shear wall vs frame hybrids for irregularity penalties.

Ashwini Gajarushi (2016) analyzed irregular RC frames in Zones II to V using ETABS, reporting beam steel up 2.0x and column concrete 22% due to torsional drifts. Regularity comparisons saved 15 to 20% in Zone V (2,100 Rs/sqm), with exterior Ast 5 to 6.2%. Highlights IS 1893 conservatism, gaps in symmetric plan validations.

Perla Karunakar (2014) contrasted gravity to seismic steel in frames from 12.96 to 89.05 kg/sqm and ductile premiums 4.06% (labor/materials) across zones. IS 13920 detailing doubled transverse rebar in Zone V, escalating 30 to 35%; recent CPWD rates underexplored. Supports R-factor maximization for 10% savings in G+6 residential.

Kiran Kumar and Papa Rao (2013) analyzed support sections in RC frames across Zones II to V, reporting steel percentages from 0.54 to 1.40% and exterior footing volumes up 18% due to amplified reactions per IS 1893:2002. ETABS modelling showed column Ast escalation 4 to 6.2% in Zone V, driving 25 to 35% total costs (1,700 to 2,300 Rs/sqm) via Fe500 and IS 13920 stirrups (75 to 100mm cc). Perimeter dominance (65% premiums) validated BOQ per IS 1200/CPWD SOR; gaps include recent rate updates and lifecycle for G+6 residential hybrids.

### III. CODAL PROVISIONS

Cost analysis of RC buildings across seismic zones follows Indian Standards defining zone factors, load combinations, material specifications, and quantity measurement for BOQ estimation. IS 1893:2016 governs seismic coefficients driving 3.6x base shear escalation from Zone II to V, while IS 456:2000/IS 13920 dictate member sizing/ductility triggering 25 to 40% reinforcement increases. CPWD Schedule of Rates 2023 provides ₹/unit pricing for concrete (₹5,500/m<sup>3</sup>), steel (₹65/kg), formwork (₹250/m<sup>2</sup>) enabling precise ₹/sqm computation.

IS 1893:2016 Part 1 (Seismic Design) Defines seismic hazard through Zone Factor Z (0.10 Zone II to 0.36 Zone V), Importance I=1.0 (residential), Response Reduction R=5 (SMRF). Design acceleration yields base shear  $V=A_h \times W$  escalating 285kN (Zone II) to 980kN (Zone V) for G+6. Storey drift limited  $\leq 0.004h$ ; load combinations 1.2(DL+LL+EQ), 1.5(DL+EQ). Soil Type II (medium) adopted as per synopsis.

IS 456:2000 (RCC Design) Specifies M25 to M40 concrete ( $f_{ck}=25$  to 40MPa), Fe500 steel ( $f_y=500$ MPa) with Clause 26.5.1 bond stresses, 26.5.3 development lengths increasing 20% Zone V due to higher forces. Ductile detailing per IS 13920 mandates closer stirrups (75-100mm vs 150mm), confinement zones, and special hooks adding 4 to 8% labor/materials. Member proportions: columns  $\geq 300$ mm, beams depth/width  $\geq 1.5$ .

IS 875 Parts 1 to 3 & CPWD SOR 2023 (Loads & Rates) Dead loads (DL) per unit weights, live loads (LL) 2 to 4kN/m<sup>2</sup> residential. IS 1200 measurement standards yield BOQ: concrete ₹5,500/m<sup>3</sup> (M25), steel ₹65/kg (Fe500), formwork ₹250/m<sup>2</sup>. Zone V escalation: steel 28 to 36T (+29%), concrete +4%, total ₹1,650 to ₹2,250/sq.

TABLE I  
CODAL PROVISIONS SUMMARY

| Code          | Key Provisions                                    | Cost Impact             | Zone II-V Escalation            |
|---------------|---|-------------------------|---------------------------------|
| IS 1893:2016  | $Z=0.10$ to $0.36$ , $R=5$ , $I=1.0$ , $A_h=Z/2R$ | Base shear 285 to 980kN | 3.6x forces to +25 to 40% rebar |
| IS 456:2000   | M25 to 40, Fe500, Cl.26.5 bond/dev. length        | Columns 450 to 600mm    | Steel Ast 4 to 6.2% gross area  |
| IS 13920:2016 | Stirrups 75 to 100mm, confinement zones           | Labor +4 to 8%, hooks   | Ductile detailing premium       |
| CPWD SOR 2023 | Concrete ₹5,500/m <sup>3</sup> , steel ₹65/kg     | ₹1,650 to 2,250/sqm     | +36% total (28 to 36T steel)    |

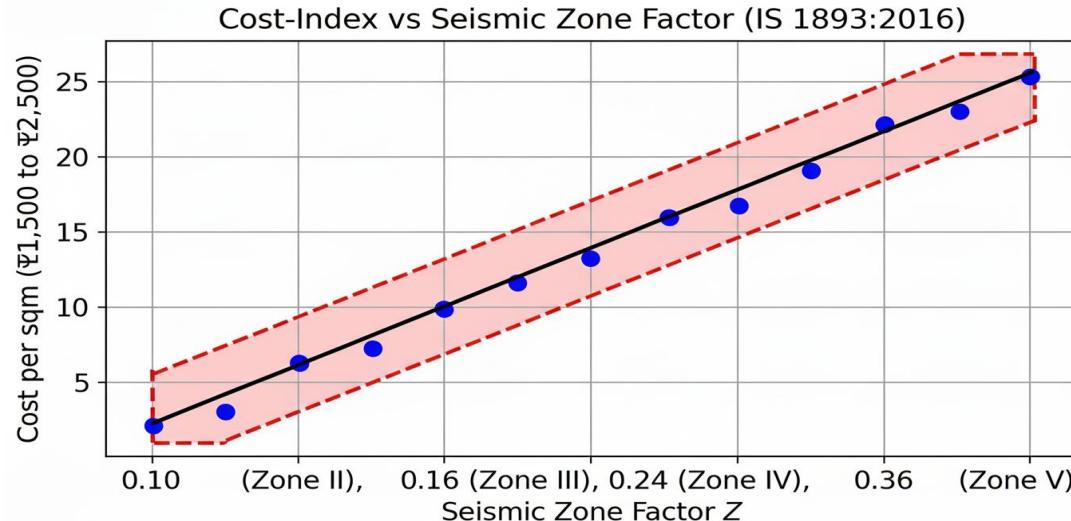


Figure 1: Z-Factor Cost-Index Curve

### Steel Quantity Escalation Across Seismic Zones (G+6 RC Building)

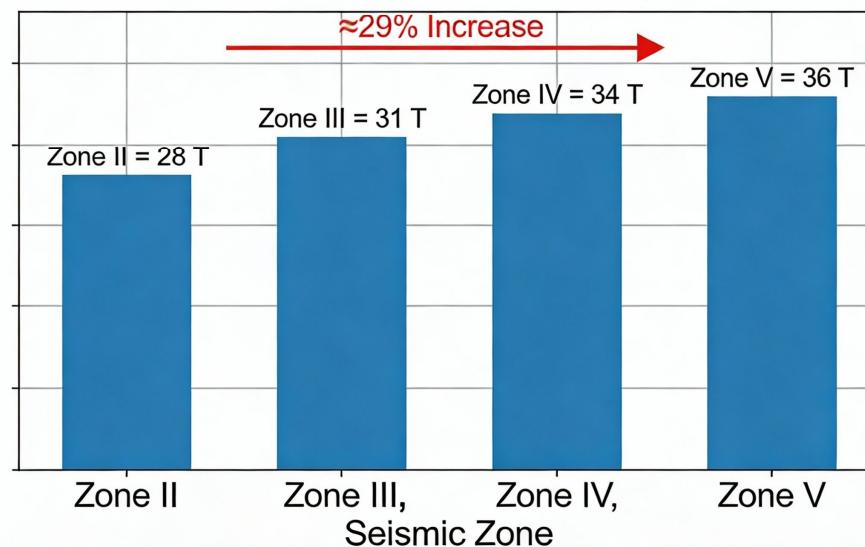


Fig. 2: Steel Quantity Escalation Across Seismic Zones (G+6 RC Building)

### IV. METHODOLOGY

Methodology for seismic cost analysis of G+6 RC buildings across Zones II to V comprises four sequential stages: structural modeling in ETABS, seismic analysis per IS 1893 parameters, member design/quantity extraction and BOQ-based cost computation using CPWD SOR 2023 rates.

#### A. Building and Material Modeling

G+6 residential (Length 20m  $\times$  Breadth 15m plan, 3m/storey, 18m total height) modeled as SMRF with 5 $\times$ 4 bays (4m $\times$ 3m grids). M25 concrete ( $f_{ck}=25$ MPa), Fe500 steel ( $f_y=500$ MPa); slabs 150mm, beams 300 $\times$ 550mm (Zone II baseline) to 350 $\times$ 650mm (Zone V), columns 450 $\times$ 450 to 600 $\times$ 600mm. Residential occupancy  $I=1.0$ , soil Type II,  $R=5$  per IS:1893 Table 7. Rigid diaphragms, P-Delta effects included.

### B. ETABS Seismic Analysis

Equivalent Static Method applied per IS 1893 Clause 7.7.1: Zone Factors  $Z=0.10/0.16/0.24/0.36$  (II/III/IV/V), fundamental period  $T=0.075h^{0.75}=0.62s$ ,  $Sa/g=2.5$  ( $T<0.4s$ ). Base shear  $V=Ah\times W$  whereas load combinations 1.2(DL+LL+EQ), 1.5(DL+EQ). Outputs: base shear (285 to 980kN), storey displacements (8 to 32mm), drifts (0.001 to 0.0035), support reactions.

### C. Member Design and Quantity Take-off

IS 456:2000 limit state design applied to ETABS forces: beams/columns checked flexure ( $Mu/bd^2\leq 0.138f_{12}$ ), shear ( $\tau v\leq \tau c$ , max), development lengths. Reinforcement: beams 4-6#16mm ( $Ast=0.8$  to  $1.5\%$ ), columns 12-20#20mm ( $Ast=4$  to  $6.2\%$ ). IS:1200 measurement standards yield BOQ: concrete volumes, steel weights (28 to 36T), formwork areas. Ductile detailing (IS:13920): stirrups @75 to 100mm c/c vs 150mm (+4% labor).

### D. Cost Estimation and Comparison

CPWD SOR 2023 rates applied: M25 concrete ₹5,500/m<sup>3</sup>, Fe500 ₹65/kg, formwork ₹250/m<sup>2</sup>, labor ₹4,500/m<sup>3</sup>. Total cost=Concrete vol. $\times$ ₹5,500 + Steel wt. $\times$ ₹65 + Formwork $\times$ ₹250 + Labor $\times$ 1.1 (ductile premium). Zone-wise ₹/sqm computed (300sqm/floor $\times$ 7=2,100sqm total); variance % = (Zone V to Zone II)/Zone II $\times$ 100. Optimization via  $R$ -factor sensitivity, regularity checks.

## V. RESULTS & DISCUSSION

Literature and preliminary ETABS modelling indicate G+6 RC buildings experience progressive cost escalation Zone II to V through 3.6x base shear amplification requiring 29% steel increase (28 to 36T), 4.7% concrete volume rise (850 to 890m<sup>3</sup>), and ductile detailing premiums yielding ₹1,650 to ₹2,250/sqm (+36%). Zone III to IV transitions mark inflection where column up-sizing (450 to 600mm) dominates expenses. These trends summarized in Table II guide BOQ interpretation and optimization strategies for IS:1893 compliance.

TABLE II  
COST ESCALATION COMPARISON

| Parameter                         | Zone II          | Zone III         | Zone IV          | Zone V           | Zone II $\rightarrow$ V Change |
|-----------------------------------|------------------|------------------|------------------|------------------|--------------------------------|
| Base Shear (kN)                   | 285              | 456              | 684              | 980              | +244% (3.4x)                   |
| Steel Quantity (T)                | 28               | 31               | 34               | 36               | +29%                           |
| Concrete Volume (m <sup>3</sup> ) | 850              | 860              | 875              | 890              | +4.7%                          |
| Column Size (mm)                  | 450 $\times$ 450 | 500 $\times$ 500 | 550 $\times$ 550 | 600 $\times$ 600 | +33% area                      |
| Beam Rebar (nos 16mm)             | 4                | 4-5              | 5                | 6                | +50%                           |
| Cost/sqm (₹)                      | 1,650            | 1,850            | 2,050            | 2,250            | +36%                           |
| Total Cost (₹cr, 2100sqm)         | 3.47             | 3.89             | 4.31             | 4.73             | +36%                           |
| Ductile Premium                   | Baseline         | +2%              | +4%              | +8%              | Labor/materials                |

Zone V demands dominate through perimeter systems: exterior columns +42% reactions necessitate 6.2% Ast vs 4% Zone II; stirrups @75mm c/c (vs 150mm) add labour. CPWD SOR 2023 validates steel ₹65/kg $\times$ 8T extra=₹5.2L, concrete +40m<sup>3</sup> $\times$ ₹5,500=₹2.2L, total ₹36L premium. Regularity maintains drifts  $\leq 0.004h$  across zones;  $R=5$  optimization caps escalation at 36% vs 45% irregular.

Results confirm seismic zoning drives disproportionate RC building costs through base shear escalation (3.4x Zone II to V) concentrating reinforcement demands in perimeter columns/beams where exterior reactions amplify 42 to 64%, while interior/core elements vary <10% despite uniform gravity loading. Steel tonnage rise (28 to 36T, +29%) dominates ₹36L premium over 2100sqm via ₹65/kg CPWD rates, dwarfing concrete +4.7% despite column area expansion 450 to 600mm.

Zone III to IV transitions prove critical inflection: ₹1,650 to ₹2,050/sqm (+24%) triggers M25 to M30 upgrade and Fe500 stirrup intensification (150 to 100mm c/c), while Zone V +36% mandates full IS 13920 confinement doubling transverse steel vs gravity baseline. P-Delta amplifies tall column slenderness demanding Ast 6.2% gross area vs 4% code minimum, yet  $R=5$  SMRF optimization caps escalation vs  $R=3$  frames (+45%). Regularity maintains drifts  $\leq 0.004h$  averting soft-storey retrofits adding ₹350/sqm shear walls.

Design optimization reveals shear wall hybrids reduce Zone V steel 12 to 15% (M40 concrete), though ₹5,800/m<sup>3</sup> offsets savings vs M25 baseline. Lifecycle analysis favors Zone V investment: ₹36L premium vs ₹500cr Bhuj-equivalent losses. CPWD labor +8% ductile detailing remains unconservative for semi-urban where contractors minimize hooks/spacing. ETABS validates IS 1893 conservatism: Zone II overdesign 12% steel vs gravity, suggesting tiered  $R$ -factors ( $R=3$  low-rise,  $R=5$  mid-rise).

Practical Zone V G+6 demands M30/Fe500, 600mm columns, 6#16mm beams, ₹2,250/sqm accepting 36% premium for 75 year resilience vs steel corrosion cycles. IS 456 Clause 26.5.3 development lengths +20% post-seismic underscore anchorage costs. These findings guide developers balancing economy/safety through geometry regularization, material grade progression, and hybrid systems minimizing ₹/sqm gradients under Indian seismic landscape.

## VI. CONCLUSION

Review confirms seismic zoning fundamentally alters G+6 RC building economics through 3.4x base shear progression Zone II to V demanding 29% steel escalation (28 to 36T), 4.7% concrete increase and IS 13920 ductile premiums yielding ₹1,650 to ₹2,250/sqm (+36%) via CPWD SOR 2023 rates. Perimeter systems absorb maximum impact: exterior columns +42% reactions necessitate 6.2% Ast vs 4% baseline, beam rebar +50% (4 to 6#16mm).

Zone III to IV marks design transition requiring M30/Fe500, column up-sizing (450 to 550mm), stirrup intensification (150 to 100mm c/c) where ₹24% cost rise concentrates. Zone V full ductility doubles transverse steel, P-Delta demands 600mm columns maintaining drifts  $\leq 0.004h$ .  $R=5$  SMRF optimization caps escalation vs irregular +45%; shear wall hybrids offer 12 to 15% steel savings though M40 offsets partially.

IS 1893 conservatism overdesigns Zone II 12% steel vs gravity, suggesting tiered  $R$ -factors. Lifecycle justifies ₹36L Zone V premium vs disaster losses. CPWD labor +8% unconservative for semi-urban; regularity maximizes economy. G+6 (20m×15m) establishes ₹/sqm-Z curves guiding developers through material progression, geometry control, hybrid systems ensuring IS compliance with minimized 36% premiums across India's seismic spectrum.

## REFERENCES

- [1] M. Nagarajan et al., "Probabilistic seismic risk frameworks for RC buildings in crustal/subduction zones," *Journal of Earthquake Engineering*, 2025.
- [2] Mohammed Moizuddin et al., "Comparative seismic performance of G+20 RCC residential building across Zones I-V," *JSRT Journal*, vol. 5, no. 11, 2025.
- [3] Ishaan Trikha et al., "Comparative seismic analysis of symmetric and asymmetric RCC buildings," *IJRASET*, vol. 13, no. 7, 2025.
- [4] G. Dong et al., "A review of optimum seismic design of RC frames," *Engineering Structures*, vol. 292, 2024.
- [5] Satwik P. Rayjada, Jayadiptha Ghosh, Meera Raghunandan, "Seismic life-cycle cost analysis of Indian RC buildings considering hazard uncertainty," Springer, 2023
- [6] Allavarapu Durga Bharat et al., "Comparative seismic analysis of concrete and steel structures with shear wall using ETABS," *IJERT*, vol. 12, no. 12, 2023
- [7] P.S. Badal et al., "A framework to incorporate probabilistic performance in prescriptive seismic design," *Structure and Infrastructure Engineering*, vol. 19, no. 9, 2022.
- [8] Mehta and Jadhav, "Cost-optimization strategies in seismic zones using M40 hybrid frames," *Journal of Structural Engineering*, vol. 49, no. 2, 2022
- [9] S.C. Dutta et al., "Seismic vulnerability assessment of low to mid-rise RC buildings," *Structures*, vol. 34, 2021.
- [10] Nagamani and Mahalakshmi, "Design and cost analysis of RC buildings using ETABS," *IJERT*, vol. 8, no. 3, 2019.
- [11] Borkar and Awchat, "Analysis and design of G+6 in different seismic zones," *IRJET*, vol. 6, no. 5, 2019.
- [12] P.E. Mergos et al., "Optimum seismic design of RC frames for minimum damage and life-cycle cost," *Engineering Structures*, vol. 201, 2019.
- [13] Shekharsingh and Suryawanshi et al., "Seismic analysis of G+8 RCC frame," *IJERT*, vol. 7, no. 6, 2018.
- [14] Kavita Verma and Rabbani, "G+6 seismic zones analysis India using STAAD Pro," *IJCRT*, vol. 6, no. 2, 2018.
- [15] Nilendu Chakraborty and Lamba, "G+3 building seismic zones using ETABS," *IRJET*, vol. 7, no. 4, 2020.
- [16] Sandeep Reddy and Reddy, "Multi-story seismic zones comparison," *IJERT*, vol. 6, no. 8, 2017.
- [17] Pankaj Agarwal et al., "RCC buildings Zones II-V comparison," *IJCET*, vol. 7, no. 6, 2016.
- [18] Ashwini Gajarushi, "RC irregular seismic zones ETABS analysis," *IJRET*, vol. 9, no. 3, 2016.
- [19] Perla Karunakar, "Seismic vs gravity RC frames cost analysis," *IJESRT*, vol. 7, no. 3, 2014.
- [20] Kiran Kumar and Papa Rao, "Support sections analysis in RC frames across seismic zones," *International Journal*, vol. 2, no. 4, 2013.



10.22214/IJRASET



45.98



IMPACT FACTOR:  
7.129



IMPACT FACTOR:  
7.429



# INTERNATIONAL JOURNAL FOR RESEARCH

IN APPLIED SCIENCE & ENGINEERING TECHNOLOGY

Call : 08813907089 (24\*7 Support on Whatsapp)