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Design of Circular Overhead Water Tank, Modelling and Analysis Using Staad-Pro

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Abstract: Water tanks are frequently used for storing potable water. Due to inadequacy of water around the world, significance is given more on the water storage project. So water storage is very essential as it plays a vital role in everyday life. water tanks and Storage reservoirs are used to store water, petroleum products, liquid petroleum, and similar liquids. All tanks are analysed and designed as crack free structures to get rid off any leakage. This project provides a brief explanation of the theory underlying the manual analysis and design of an overhead circular water tank using the Working Stress Method (WSM) and software modeling and analysis using Stat-Pro.

Keywords: Overhead Circular water tank, Staad-Pro, Population Forecsting Method, Limit State Method, Working State Method, IS Code

I. INTRODUCTION

Water is an essential necessity for human survival, and the distribution of enough water in a given region depends on the water tank's design. An elevated water tank is a type of water storage container used to maintain a water supply at a height high enough to pressurize a water distribution system. They were swept away by the wind. Damage of the important lifeline facility like elevated water tanks often results in significant hardships even after the occurrence of the disaster, claiming human casualties and economic loss to build environment. Investigating the effects of wind has been recognized as a necessary step to understand the natural hazards and its risk to the society in the long run. Because water towers rely on the elevation of water caused by gravity and hydrostatic pressure to force the water into home and industrial water distribution systems, they can provide water even when there is no electricity.

A. Objective

- *1)* To make a study about design of water tank.
- 2) Design of Circular Overhead Water Tank by WSM and other staging members by LSM Method.
- 3) To make a study about the Analysis of Water Tank using Staad-Pro.
- 4) To make a study about the guidelines for the Design of Liquid retaining Structures according to IS Code.

II. METHODOLODY

In this project we have designed a Water Tank Manually and same analyzed on Staad-Pro software.

A. Data Collection

Details of data collection

1.	Capacity of tank	140000 L.
2.	Soil bearing capcity	125 KN/sq.m at minimum depth 3.00m
3.	Height of tank from ground	12m
4.	Grade of steel	fe500
5.	Grade of concrete	M30



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	Ground water table	Maximum height of water table is assumed to
		be well below the foundation strata level
		hence no uplift pressure has been considered
		in the design footing.
6.	External force on tank	Basic wind speed 39m/s
7.	Free board	0.2m
8.	Width of gallery	1m
9.	Earthquake zone	III
10.	Use of water	Domestic purpose only
11.	Current population in year	2480
	2011	
12.	Population forcasting 2041	3500

B. Population Forcasting

We have forecasted the population by using Arithmetic Mean Method. We have collected the data from our proposed site and received data from 1979. Has been forecasted till 2041 by Arithmetic Progression.Population Forecasted is 3500 no of people.

III. DESIGN CRITERIA

A. Loads

- Dead Load: The weight of all permanent construction including domes, ring beams, walls, stair case, slabs and foundation are considered. The unit weights of materials are in accordance with IS: 875-1987. The unit weight of Concrete (RCC), Soil,, Structural steel is taken as 25 kN/m3 ,18 kN/m3 ,78.5 kN/m3.
- 2) Live Load: The Live load on roof slab, walk way slab and staircase be 1.5 kN/m^2 , 1.5 kN/m^2 and 2.0 kN/m^2 respectively.
- Water Load: Weight of water due to gross volume is calculated and applied on bottom of container unit wt. of water is 10 kN/m³
- 4) Wind Load: As per figure -1 IS: 875(PART-3)-1978) design wind pressure = $0.6Vz2 = 2117.01 \text{ N/m}^2$
- 5) Earthquake load (EQ)
- It is in zone-III as per IS 1893 part1 2002
- Seismic coefficient $\alpha h = \beta IFo(Sa/g)$
- $\boldsymbol{\beta}$, coefficient of depending upon soil foundation = 1
- I, factor depending upon importance of factor = 1.5
- So, seismic zone factor for average acceleration spectra = 0.16

Sa/g is considered as per CI 6.3.5,(IS 1893,part-1).

B. Structural design of RCC OHSR

Calculations

1) Dimension of Tank

- Diameter Of Tank D = 7.2m
- Radius= 3.6m
- Height =4m
- Thickness Of Tank Wall =170mm

2) Wind load calculations
By using IS 875 Part3 1987 Appexndix A.
Basic Wind Speed - 39 m/s..Table 1
Pz =0.6 vz² - Design wind speed.
Pz = 0.527 kN/m²



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3) Seismic Load Calculations Using IS 1893 Part-1-2016 as the site exist in zone 3 all the parameters taken according to that.

Total Seismic weight on structure, =4102.04+4102.04+4071.50 =12275.58KN

- 4) Circular Slab (Top roof) Diameter D =7.2m Thickness=170mm L.L =1.5 Kn/m² M30 And Fe 500
 - Total Load = 5.75 Kn/m^2
 - Max BM = -9.315 Kn.m

dreq = $\sqrt{(M/0.133 \text{ fckb})}$ = $\sqrt{(9.315 \times 10^6/0.133 \times 30 \times 1000)}$ dreq =48.31mm Provide D =150mm Using 10mm bar and clear cover 15mm d=150-15-10/2=130mm for bottom layer And d=130-10=120mm for top layer

• steel reinforcement at centre (provide in mutually parallel direction)

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Ast = 0.5 fck/fy [1-\sqrt{1-(4.6Mu)/(fckbd^2)}] bd
Ast = 206.77 mm<sup>2</sup>
Ast min = 0.12/100 \ge 1000 \ge 170
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 $= 204 \text{mm}^2$

Provide min. reinforcement Using 10mm bar Spacing= 1000ast/Ast =380mm Max. Spacing=3d= 3 x 120 =360mm or 450mm }less Provide mesh reinforcement consisting of 10mm ø 360 c/c in mutually both direct at as bottom reinforcement.

At edge = no reinforcement $(M\phi)e=0$

- 5) Design of Top Ring Beam
 - Load Of Slab = 4.25 Kn/m^2
 - For hoop force
 - T= w x R x (cosø 1/ Hcosø) = 10.35kn/m
 - Hoop stress = $10.35 \times 10^3/1000 \times 170$
 - = 0.06 <8N/mm² (8 N/mm² = permissble stress in concrete)Table 2, IS 3370 (part 2): 2009
 - It is designed for hoop tension -

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W = T \cos \phi = 10.35 \text{ x} \cos \theta = 10.35 \text{ KN/m}
Total hoop tension in beam
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W x D/2= $\frac{10.35 \text{ x7.2}}{2} = 37.26 \text{ kN}$ AST for hoop tension T/ σ_{st} = 37.26 x 10³/130 = 286.61 mm ² [σ_{st} = 130N /mm ² Table 4, IS 3370 (part 2):2009] σ_{st} = permissible stress in steel reinforcement use 12 mm Ø bar To find out dimension of ring beam $\sigma_{ct} = T / Ag + (m-1) Ast < 1.5$ $Ag = b \times D$. Assume b=230 mm $m = 280 / \sigma_{cbc} = 280 / 3x10 = 9.33$ σ_{cbc} = permissible stress in concrete [Table2, page3, IS 3370(Part 2): 2009] $\sigma_{ct} = 37.26 \times 10^{3}/230 \times D + (9.33-1) \times 286.61 < 1.5$ $=37.26 \text{ x}10^3 < 345\text{D} + 3581.19$ = 33678.808/345 < D = 97.61<D Consider D= 350mm Provide minimum shear reinforcement 6) Design of Tank Wall Max. hoop tension at base, $T = (\gamma_w \times H \times D) \div 2$ $= (10 \times H \times 7.2) \div 2$ =72H÷2 T=36H KN/m Ast= $T \div \sigma_{st}$ =(36÷130)×10³ =276.92H mm² To find thickness of wall:- $\sigma_{ct} = T \div [Ag + (m-1)Ast]$ T=36×4=144 KN $\sigma_{ct}=1.5$ Ag=1000 \times t (m-1)=9.83-1=8.3 m/s calculated at ring beam $\sigma_{ct}=(144\times10^3)\div[1000\times t+(8.33\times2\times565.45)]<1.5$ $=(144\times10^{3})\div(1000t+9420.397)<1.5$ 144×10³<1500t+14130.595 86.57<t Provide t=200m at base and 150mm at top Average thickness of wall =175mm Distribution Steel:- $H \div 3 = 4 \div 3 = 1.33 m$ Cantilever moment (m):- $M = [\gamma_w \times H \times (H/3)^2] \div 6$ $=[10 \times 4 \times (1.33)^{2}] \div 6$ M=11.84KN-m Ast for moment= $M \div (\sigma st \times j \times d)$ J=1-(K \div 3) from the base slab d=175-50=125mm

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Ast for moment=(11.84×10⁶)÷(130×0.85×125) =857.19mm²

7) Design of Base Slab

H =4m D =7.2 R=3.6m Assume thickness=250mm 1) load ($/m^2$ area) a) weight of water = $10 \times V$ = 10 x 1 x 4= 6.25 KNTotal = 40 + 6.25 = 46.25 KNAccording IS 3370, part 4 table 20 pg 47 Moment= coefficient X PR² (Kgm/m) At R = 0, at centre $Mr = Mt = +0.075 X 46.25 X 3.6^{2}$ = +4495KN.m At support, where R=R Mr= -0.125 x 46.25 x 3.6² = -74.92KN.m $Mt = -0.025 \times 46.25 \times 3.6^2$ = -14.98KNm

Check for d (to check if dpro > dreq) dreq = $\sqrt{Mmax} / R x b$ = 74.92x10⁶/1.618x1000 (R is design constant) dreq = 99.27 mm dpro = depth of base Slab 250 mm clear cover= 25 diameter=20 dprv = 250-25-20/2 = 215mm > dreq

1] Ast for - Mr at support

Ast = M/Sigma St x j x dprov = 74.92 x $10^{6}/115 x 0.85 x 215$ = 3564.8 mm Assume dia.= 20 mm n = Ast/ ast = 3564.8/ $\pi/4 x 20^{2}$ n = 6.80 = 7 Spacing= 1000/n =1000/7= 142.85=150 mm c/c.

Check for shear $V = P \ge 2R / 2 = 46.25 \ge (2 \ge 3.6) / 2 \dots R = Radius$ V = 166.5KNTau v = V/ bd_{prov} = 166.5 \x 1000/ 1000 \x 215 100 Ast / bd_{prov} = 100 \x [1000 \x (\pi / 4) \x 20²]/ 1000 \x 215 = 1.46 $\tau_{C} = 0.4436 \ N/mm^{2}$ $\tau_{v} > \tau_{C}......Not safe$ International Journal for Research in Applied Science & Engineering Technology (IJRASET)



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Provide a haunch of 20 mm size at the junction.

8) Design of Brace Beam

Assuming Beam Size Square beam = 300x300 mm Length between bracing= 3.8mTotal load = 2.5 + 2.25 + 5.375 = 10.125Kn/m Effective depth= 300-50 = 250mm Load calculation= Design load = 10.125×1.5 = 15.187KN/m Moment calculation= Mu = wxL²/8 = $15.187 \times (3.8)^2/8$ = 27.41KNm $M_{ub} = 0.138 \times fck \times bd^2$ = 111.78×10^6 KNm.

Mub>Mu.....hence singly reinforced beam

- 9) Design of Bottom Ring Beam
 - Total vertical load = v_1 = 31.875 KN/m.
 - Horizontal force = $H=v_1 \cos \phi = 31.875 \text{KN}$
 - Hoop tension due to vertical loads-
 - Hg = H X D/2 =26.775 X 7.2/2 = 96.39 KN.
 - Hoop tension due to water pressure
 - Hw= w x h x depth of beam x dia /2 =10 x4 x 0.350 x 7.2/ 2. = 50.4 KN
 - Total hoop tension = Hg + Hw = 96.39 + 50.4 = 146.79 kN
 - Ast = $146.79 \text{ x } 10^3/150 = 978.6 \text{ mm}^2$

10) Design of Column

6 Columns equally spaced on 7.2m diameter circle Height of Column=12m Distance between Columns=3.8m Diameter of Column=300mm Pu=1095.845KN Condition:-Column is effectively held in position. and restrained against rotation in both ends. Leffective =6m Slenderness ratio =20mm>12mm Minimum Eccentricity: emin=34mm>20mm

Area of Reinforcement(Asc):-Ag= $(\pi \div 4) \times (300)^2 = 70685.83 \text{ mm}^2$ Ac = 70685.83-Asc Puz=0.4fck.Ac + 0.67fy.Asc



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1095.845×10³ = [0.4×30×(70685.83-Asc)]+ (0.67×500×Asc) Asc=766.61mm² Assume 5% of steel, Adopt 12mm Ø bar

Helical Reinforcement:-Assume cover 40mm Core Diameter(dc)=220mm Area of Cover(Ac)=38013.27mm² P=Pitch of Spiral ties Vc=Volume of core =38013.27×P Using 10mm & spirals (helical reinforcement) Provide pitch of 30mm

11) Design of FootingArea of Footing:-As per IS code guidelines self wot of footing to taken 10% of column loadAs load carried by column is 1095.85KNNote: Design of square footing is done exactly in the same manner as PL was for square column

Side of square column:b=0.717D=0.717×300=215.1≈220mm Note- As per IS recommendation for the purpose of design of footing for circular column the size of column is taken as 0.7170 times dia.

Area of Footing=Load ×SBC =1205.4295×125.12 SBC is taken as per test results on site 1205.4295÷125.12 =9.63 m² Side of square footing $B=\sqrt{9.63}=3.10=3.2m$

Therefore size of square footing for circular column B x B= $3.2 \times 3.2 \text{ m}$. Net upward pressure (Po) =117.71KN Step-Depth of Footing on the basis of BM M=Po(B÷8)(B-b)² Now, Ultimate moment (Mu):-Mu= $1.5 \times M$ = 1.5×395.97 Mu=593.955KN-m

Dreq

d=215.68mm≈220mm

 $Ast = 3582.34 \text{ mm}^2$

Footing has been checked for one way and two way shear, it has been safe.



C. Modelling on Staad-Pro







Created model by using material properties



Dead Load Application





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Bending Moments



Torsional Moments

IV. CONCLUSION

- 1) The tank has been analyzed in STAAD Pro software.
- 2) In terms of the loads used, tank design is safe from software design.
- 3) We use circular water tanks for larger capacity while rectangular water tanks are used for smaller capacities. Since our proposed tank is of 1.40 lakh capacity we had prepared and analyzed the circular over head tank in STAAD Pro software.
- 4) Design of water tank is also done by manually by WSM and LSM method.
- 5) Design of water tank by manually and by using software are within limit and safe.

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IS code

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