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## Seismic Control of a Concrete Bridge with Hybrid Passive Control Devices

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Abstract: This study focuses on the seismic response of a three-span deck slab RCC T-Girder bridge with and without LRB (Lead Rubber Bearing) and in combination with LRB and FVD (Fluid Viscous Damper). The bridge is subjected to AASHTO, IRC design standards, and IRC Class A vehicle load. Linear time history analysis using CsiBridge and SAPfire is conducted to assess the bridge's seismic behavior. Excessive deck displacement, which can lead to deck failure and bridge closure, is a key concern. The study utilizes finite element analysis with Csibridge and incorporates viscous dampers throughout the bridge. The findings show that the presence of supplemental dampers significantly reduces pier top displacements. Base shear, deck displacement, pier response, and structural time period are the primary response parameters examined. The effectiveness of different isolation system configurations is compared to the non-isolated bridge in terms of these response characteristics. The study offers suggestions for design improvements to enhance the structure's performance.

Keywords: RCC T-Girder bridge, LRB and FVD, CSiBridge software, IRC class vehicle, Ground Motion Records, Linear Time History.

#### I. INTRODUCTION

This paper explores the use of hybrid passive devices, including elastomeric bearings and viscous dampers, in RC bridges to enhance seismic behavior. Upgrading bridges is vital to mitigate collapse and lengthy closures due to excessive deck displacement. Base isolation and dampers effectively improve seismic performance, allowing non-structural components to withstand earthquake forces. While elastomeric bearings can resist minor vibrations, dampers optimize structural performance during strong seismic events. Energy dissipation devices like laminated rubber bearings increase damping and reduce displacement demands. The study emphasizes the need for strong, flexible, and deformable structures to dissipate transient inputs. Additional dampers absorb energy, reducing stress and deflection. The research focuses on fluid viscous dampers using linear time history analysis for bridge applications.

#### II. LITERATURE REVIEW

- M. C. Kunde and R. S. Jangid [1]. (*Effects of Pier and Deck Flexibility on the Seismic Response of Isolated Bridges*.) A study analyzed isolated bridges' seismic response using elastomeric bearings and sliding mechanisms with real earthquake ground motions. Different mathematical models yielded similar results, emphasizing the effectiveness of isolation systems in reducing earthquake impact on bridges.
- 2) Seyed Saman Khedmatgozar Dolati, Armin Mehrabi and Seyed Sasan Khedmatgozar Dolati [2]. (Application of Viscous Damper and Laminated Rubber Bearing Pads for Bridges in Seismic Regions)Study investigates the use of VD-LRBPs (viscous dampers and laminated rubber bearing pads) in reducing bridge displacement during earthquakes. Results show VD-LRBPs effectively decrease residual displacement, facilitate self-centering, minimize force transfer to the foundation, and dissipate seismic energy. Promising potential for seismic zones.
- 3) Li Zhen, Li Dejian, Peng Leihua, Lu Yao, Cheng Kepei and Wu Qianqiu[2]. (Study on the damping efficiency of continuous beam bridge with constant cross-section applied by lead rubber bearings and fluid viscous dampers.) Modifying seismic properties impacts multi-span bridge responses. Achieving effective isolation in both horizontal directions is challenging. Lead rubber bearings provide insulation and energy absorption, while fluid viscous dampers dissipate seismic energy. However, lead rubber bearings have limited horizontal resistance under severe earthquake loads due to decreasing stiffness after yielding.
- 4) CSiBridge Bridge Superstructure Design IRC-2011[6]. CSiBridge is a comprehensive software for bridge modeling, analysis, and design, capable of handling various bridge types. It automates design tasks based on load patterns, but users need to verify results and address other design aspects not covered by the software. Results can be visualized graphically and printed with customized reports, enhancing the bridge superstructure design process.



5) IRC 2011, IS 1893, and IRC 84 ,IRC83 & SP-114(2018). The literature review covers four Indian codes relevant to highway and bridge design. IRC 2011 guides highway bridge design with earthquake-resistant provisions. IS 1893 is the standard code for seismic design, including seismic zones and analysis. IRC 84 provides rural bridge design standards, including earthquake resilience. IRC 83 focuses on elastomeric bridge bearings, detailing material properties, design criteria, testing, and inspection.

#### III. LAMINATED RUBBER BEARING AND FLUID VISCOUS DAMPER

High density rubber bearing (HDRB) is another type of elastomeric bearing which consist of thin layers of high damping rubber and steel plates in alternate layers. The rubber used is either natural rubber or synthetic rubber which provide a sufficient amount of damping.



 $F_b = c_b \dot{x_b} + k_b x_b$ 

c b and k b represent damping and bearing stiffness. The mentioned damping and stiffness in a LRB system have specified values of isolation period (T b) and the damping ratio ( $\xi b$ ). F b the restoring force, x b the relative base displacement, x b the relative base velocity. A spreadsheet is evaluated for getting the MoI in order to get response from bearing.

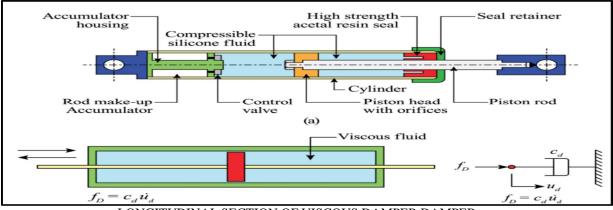
Support Proper	ty Data	COMPANY.	
Link/Support Typ	e Rubber Isola	- note	
Property Name	LBB1		Set Default Name
Property Notes			Modify/Show
Total Mass and W	/eight		
Mass	0	Rotational Iner	tia 1 1.82E+10
Weight	0	Rotational Iner	tia 2 0
		Rotational Iner	tia 3 0
Factors For Line, A	Area and Solid Sprin	ngs	
Property is Define	ed for This Length Ir	n a Line Spring	1
Property is Define	ed for This Area In A	vrea and Solid Springs	1
Directional Proper	ties		P-Delta Parameters
Direction Fixe	ed NonLinear	Properties	Advanced
V1 [		Modify/Show for U1	
IV2 □	- Y	Modify/Show for U2	
U3/ 🗆	- 🔼	Modify/Show for U3	1
E B1 E			

CSIBRIDGE INTERFACE FOR DATA FEEDING FOR BEARING

C45	• : × 🗸	$f_x = IF(0)$	G5="squ	are",C41^4/12	2,PI()/64*C41	_^4)									
A	В	С	D	E	F	G	н	I.	J	К	L	м	N	0	P
1				Isolator Prop	perties										
3	Elastomer Propertie	<u> </u>			De	Design Assumption						INPU			
4	Units:		KN, mm	Un	its:		U	Jnits:	KN, mm				Cell C	ontains	AUT
5	Shear Strain (%)	50	50	Plan Shape		Circular	Maximum A	pplied Dis., Dm	351.8						
5	Shear Modulus, G	0.707	0.00071	Plan Dimension	(d-B)	800	Structure Da	mping, β (%)	5						
	Ultimate Elongation	5	5	Rubber Cover T	ĥ.	10									
5	Material Constant, k	0.7	0.7	No. of Rubber L	ayers	18									
	Elastic Modulus, E	2.63	0.00263	Rubber Layer T	hickness, tr	9									
0	Bulk Modulus, E <sub>∞</sub>	1000	1	No. of Mid. Stee	el Pl.	17									
1	Damping	0.05	0.05	Mid. Steel Pl. Tl	hickness, ts	2			Vertica	al Stiffi	ness (K	N, mm	)		
2	Lead Yield Strength, opl	8	0.008	Lead Core Diam	eter, d <sub>pl</sub>	140		Bonded Din	nension				780		
3	Gravity	9810	9810	Total Height		196		Bonded Are					477830	5	
4		U				V		Bonded Per				-	2450		
5								Shape Facto				-	21.0	_	]
7	Bearing Properties	(KN, mm)						Moment of	Inertia, I				.817E+	10	1
8	Gross Area, A.		655					Reduced Are	- CDubb			-	696 213042	2	
2	Plug Area, Aol		394								F	-	1.622	_	
	Rubber Area, A <sub>r</sub>	487						Effective Compressive Modulus, E <sub>e</sub> Vertical Stiffness of an Internal Laver, K <sub>ei</sub>					-		
0	Total Rubber Thickness, Tr	407							Vertical Suffness of Isolator, K <sub>vi</sub>		2132			-	



1) Viscous Dampers: The Viscous dampers are passive energy dissipation device which is added to structure to increase the effective stiffness of new and existing structures. They are very robust material and energy is transferred by piston and absorbed or vanishes by silicone-based fluid flowing between the piston and cylinder arrangement. The properties of the Viscous Damper are considered as provided by the manufacturing company Taylor Device Inc. The force and weight value of the FVD500 is interpreted by the following literature made available by company and fed into the software.



LONGITUDINAL SECTION OF VISCOUS DAMPER DAMPER

FORCE (kN)	TAYLOR DEVICES MODEL NUMBER	SPHERICAL BEARING BORE DIAMETER (mm)	MID- STROKE LENGTH (mm)	STROKE (mm)	CLEVIS THICKNESS (mm)	MAXIMUM CLEVIS WIDTH (mm)	CLEVIS DEPTH (mm)	BEARING THICKNESS (mm)	MAXIMUM CYLINDER DIAMETER (mm)	WEIGHT (kg)
250	17120	38.10	787	±75	43	100	83	33	114	44
500	17130	50.80	997	±100	55	127	102	44	150	98
750	17140	57.15	1016	±100	59	155	129	50	184	168
1000	17150	69.85	1048	±100	71	185	150	61	210	254
1500	17160	76.20	1105	±100	77	205	162	67	241	306
2000	17170	88.90	1346	±125	91	230	191	78	286	500
3000	17180	101.60	1441	±125	117	290	203	89	350	800
4000	17190	127.00	1645	±125	142	325	273	111	425	1088
6500	17200	152.40	1752	±125	154	350	305	121	515	1930
8000	17210	177.80	1867	±125	178	415	317	135	565	2625

SECTION DETAILS OF VISCOUS DAMPER DAMPER

Link/Support Type	Damper - Exponential		P-Delta Paramete     Shear Coupl	
Property Name	FVD500	Set Default Name	Equal End M	oments
Property Notes		Modify/Show	Advanced	Modify/Show
otal Mass and Weig	pht			
Mass	9.806E-03	Rotational Inertia 1	C	).
Weight	500.	Rotational Inertia 2	c	

CSIBRIDGE INTERFACE FOR VISCOUS DAMPER DAMPER DATA

#### IV. CSIBRIDGE SOFTWARE.

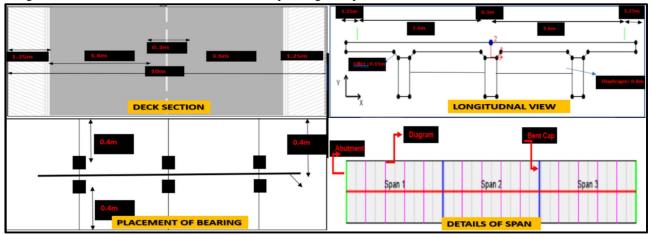
CSiBridge is a powerful bridge analysis and design software that uses a parametric object-based modeling approach. It simplifies the creation of bridge systems by allowing designers to assemble and assign bridge components easily. The software's SAPFire Analysis Engine automatically converts the model into a mathematical finite-element model, enabling structural optimization and cost-saving designs. CSiBridge enables quick generation and analysis of vehicle loading scenarios, enhancing design checks, especially for steel/composite bridge decks. Its influence-based enveloping analysis is particularly useful for analyzing moving vehicle loads.

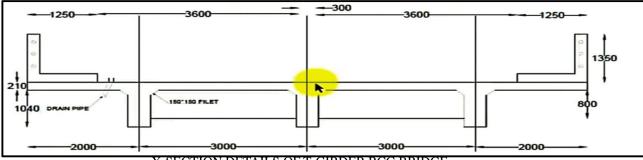


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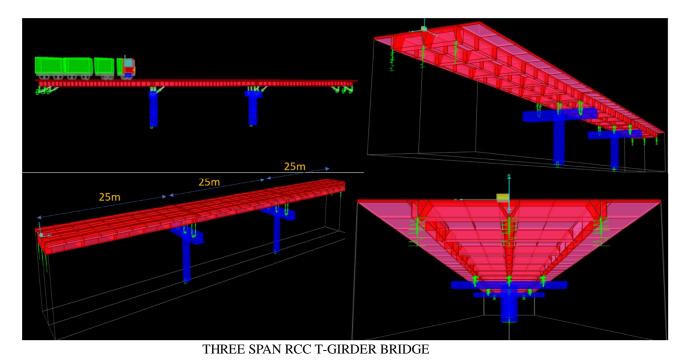
#### V. TYPES OF LOADS/CASES ON BRIDGE AND SECTION DETAILS

The software automatically calculates and takes into consideration the self-weight of the three girders and the deck section that are represented. According to IRC 6, the carriageway width of the Benchmark bridge should be adopted for bridges with carriageway widths between 5.3m and 9.6m. This corresponds to either one lane of Class 70R or two lanes of Class A vehicle loading. As a result, the bridge now has two lanes of Class A vehicle load operating at a speed of 50 km/hr. The section Details are shown below :





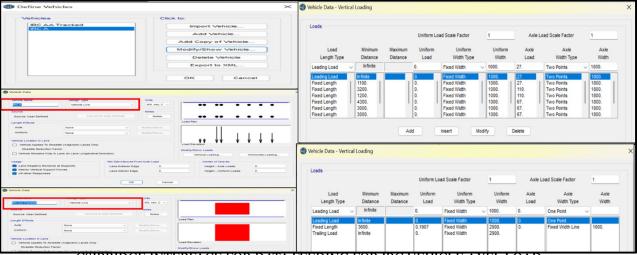
X-SECTION DETAILS OF T-GIRDER RCC BRIDGE





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CSIBRIDGE INTERFACE FOR DATA FEEDING FOR IRC VEHICLE AXEL LOAD

Define Load Cases			X	Define Load Patterns						Х
Load Cases Load Case Name	Load Case Type		Click to: Add New Load Case	Load Patterns		Self Weight	Auto Latera	al	Click To:	
DEAD MODAL CLASS A	Linear Modal History Modal Linear Multi-step Static		Add Copy of Load Case	Load Pattern Name CLASS A	Type Vehicle Live	Multiplier	Load Patter	n ~		v Load Pattern
MOVE1 eicentroloadcase_ns eicentroldcase_ew	Moving Load Linear Modal History Linear Modal History		Modify/Show Load Case	DEAD Barrier Sidewalk	Dead Dead Dead	1			Modify	Load Pattern
		•	Display Load Cases	DW Temperature Pos	Wearing Surface Temperature	0				ldge Live Load
			Show Load Case Tree	CLASSA	Temperature Vehicle Live	0				d Pattern Notes
		]	OK Cancel							
Load Case Data - Linear Multi-step Static			×	Multi Step Bridge Live Loa	d Pattern Constation	1			ОК	Cancel
CLASS A Set 0	Def Name Modify/Show		ad Case Type Iuti-step Static V Design	Walti Step Bruge Live Loa	a ratem deneration	1				^
Stiffness to Use     Zero Initial Conditions - Unstressed State			Linear Nonlinear	Vehicle	Lane	Start Dist	Start Time	Direction	Speed	Location
O Stiffness at End of Nonlinear Case Important Note: Loads from the Nonlinear C case	ase are NOT included in the current		ISS Source ISSSRC1		NE2 V				5000.	V
Loads Appled Load Type Load Name Load Pattern V CLASS A	Scale Factor			IRC AA Tracked LA IRC A LA	NE2 NE1	D			5000. 5000.	
Load Pattern CLASSA	1. Add									
	Modify Delete									

CSIBRIDGE INTERFACE FOR LOAD CASES

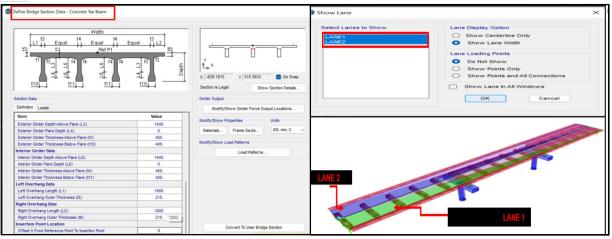
#### VI. PROCESS OF MODELLING OF THE BRIDGE

The following process in Brief is as followed: -

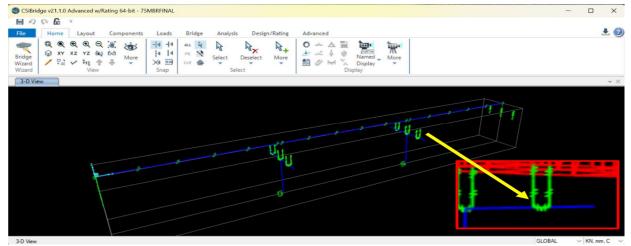
- 1) Creating Layout Line: It is used for defining the horizontal and vertical alignment of the bridge and the vehicle lanes. The bridge under modelling is a straight bridge 3span of length 25m c/c of abutments and pier. So, the initial station was set to 0 and the end station to 75m, with no change in vertical layout data.
- 2) Assigning Material Property: M25 grade concrete is used for the construction of deck slab and girder.
- 3) Frame Sections: The frame section properties of the precast girders are defined by entering their sectional dimensions.
- a) Deck Section: The dimensions of the bridge superstructure are defined and the precast concrete girder is used.
- *b) Bearing and Damper Properties:* Bearing property is user-defined and allows each of the six degrees of freedom to be specified as fixed, free or partially restrained with a specified spring constant. The bridge is simply supported, so hinge and roller bearing properties are assigned.
- c) Bridge Object Modelling: In this, all the data regarding the bridge such as the number and length of spans, location of its abutments in terms of its elevation and bearing are added.
- d) Bridge Lane Data: Lanes must be defined to analyse the bridge for vehicle live loads.
- e) Vehicle Data: Vehicle loads are applied to the structure through lanes.



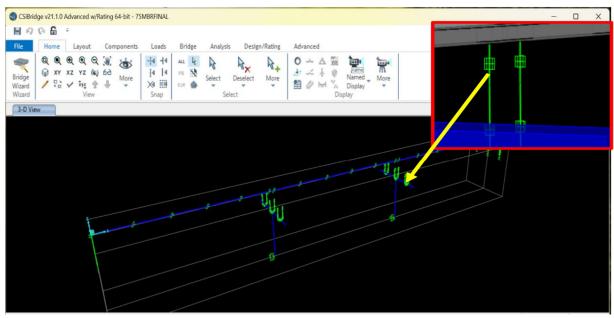
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X-SECTION DETAILS OF T-GIRDER RCC BRIDGE



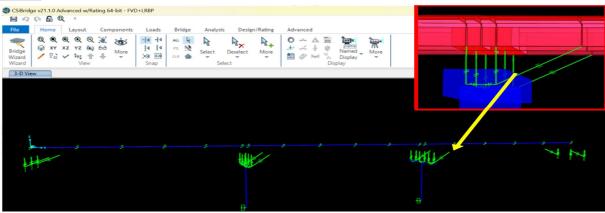
CSIBRIDGE INTERFACE FOR LINEAR LINK



CSIBRIDGE INTERFACE FOR LRB



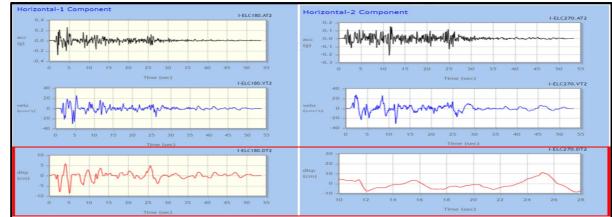
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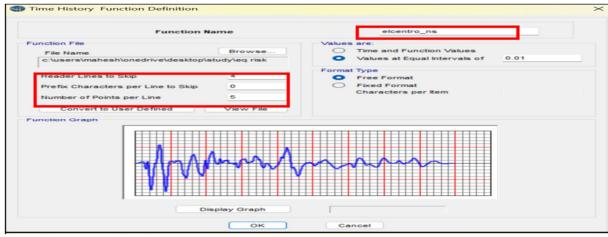
CSIBRIDGE INTERFACE FOR LRB + FVD

#### VII. ANALYSIS PROCEDURE

In this study, a three-span continuous deck slab bridge is analyzed using linear time history analysis. The bridge is subjected to displacement ground motions in the transverse direction. The El Centro 1940 ground motions, scaled to 0.4g, are used for the analysis. The CSiBridge program is employed to calculate the maximum and minimum response quantities by utilizing influence-based enveloping analysis. The program automatically places vehicles on the bridge lanes to generate influence surfaces and lines, allowing for effective analysis of vehicle loads and validation of the design. Response quantities such as shear force, bending moment, deflection, axial force, and torsion are evaluated using this approach.



NS & EW COMPONENTS OF EL CENTRO GROUND MOTION [Pacific Earthquake Engineering Research]

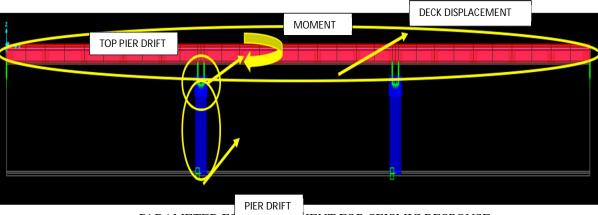


CSIBRIDGE INTERFACE FOR TIME HISTORY FUNCTION



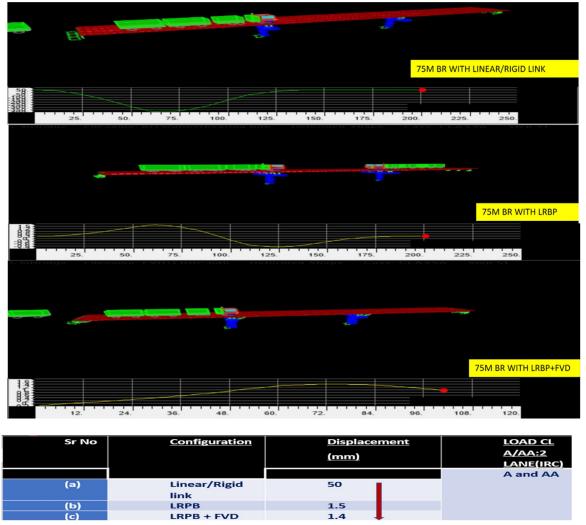
#### VIII. RESULTS FOR A THREE SPAN RCC BRIDGE

The parameter for assessment of results are as under :-



PARAMETER FOR ASSESSMENT FOR SEISMIC RESPONSE

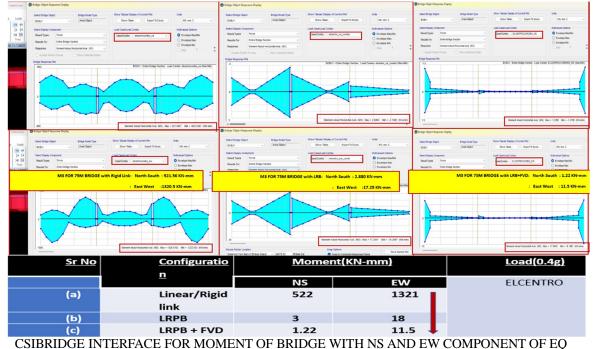
1) Class A/AA: Br Transverse Displacement. By combining LRB with FVD, the bridge's ability to resist transverse displacement is enhanced compared to using LRB or rigid link alone. The added damping properties of FVD contribute to reducing the overall transverse displacement of the bridge during seismic events.

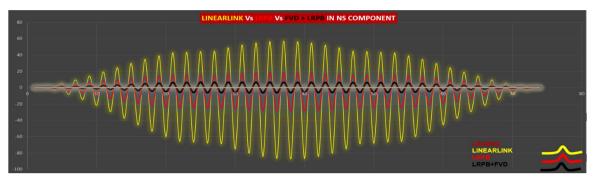


CSIBRIDGE INTERFACE FOR DYNAMIC RESPONSE OF BRIDGE WITH IRC VEH LOAD

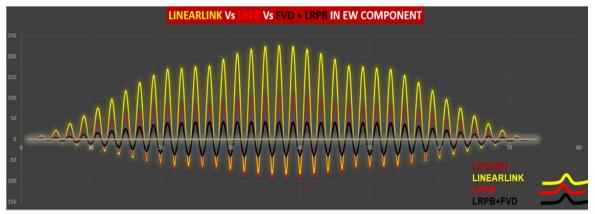


2) Force/Moment about Horizonal Axis under scaled ground motion. The application of Laminated Rubber Bearings (LRB) reduces the moment response of a bridge by providing flexibility and isolation. When combined with Fluid Viscous Dampers (FVD), the moment response is further reduced due to additional damping and energy dissipation. The combination of LRB and FVD enhances the bridge's ability to resist moment forces and improves its seismic performance.





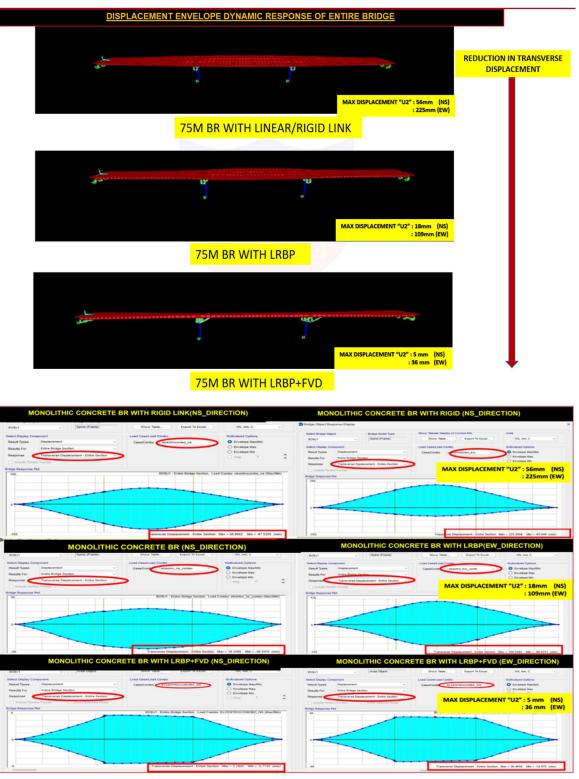
CSIBRIDGE INTERFACE FOR DECREASED DISPLACEMENT OF BRIDGE WITH NS COMPONENT OF EQ



CSIBRIDGE INTERFACE FOR DECREASED DISPLACEMENT OF BRIDGE WITH EW COMPONENT OF EQ



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CSIBRIDGE INTERFACE ILD FOR DECREASED DISPLACEMENT OF BRIDGE

3) Dynamic Response of Pier. when Laminated Rubber Bearings (LRB) are used in bridge piers, they can potentially increase the shear response. However, the moment response and axial forces of the piers is effectively reduced due to the flexibility and rotational capacity provided by LRBs. When LRBs are combined with Fluid Viscous Dampers (FVD), the overall response of the piers in terms of both shear and moment can be controlled and mitigated effectively.

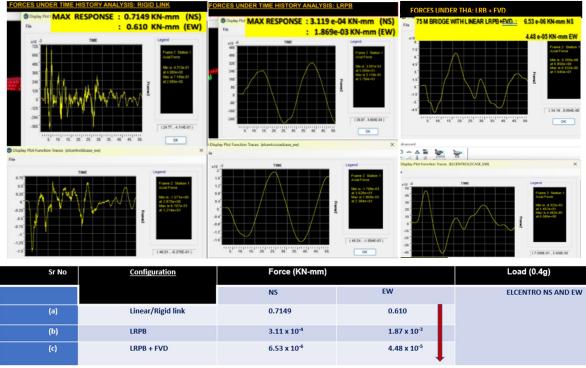


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Diagram	s for Frame Object 36(PI				WITH LINEAR LINK : NS (M) 4501KN
Case	elcentrocombo_ns		nd Length ( .ocation)	Jt:	NS (S) 783 KN
Items	Major (V2 and M3) V Ma	x/Min Env 🗸	I-End:	0. (0. m)	N3 (3) 103 KM
			J-End:	Jt: 59 0.6 m	
Resultant	Shear			(6.2 m)	
Resultant	Shear	1			Shear V2
					783.113 KN at 3.1 m
					-1251.874 KN at 3.1 m
Resultant	Moment				Moment M3
					4501.0367 KN-m
					at 0. m -7295.4087 KN-m at 0. m
1					at 0. m
Diagr	ams for Frame Object 36 (	nier)			WITH LRPB : NS (M) 1003 KN-M
e lugi		p	F- 41		WITH LRPB : NS (M) 1003 KN-M
Case	elcentro_ns_combo		End Length (Location)	Jt	NS (S) 0404 KN
Items	11	Max/Min Env. 🗸	I-End:	0 (0. m)	NS (S) 2424 KN
			J-End:	Jt: 59 0.6 m	Location
				(6.2 m)	6.8 m
Result	ant Shear				Shear V2
					2424.929 KN
					-1673.148 KN at 6.8 m
Result	ant Moment				
				1	Moment M3
					1003.8886 KN-m -1454.9574 KN-m
					at 6.8 m
Diagra	ams for Frame Object 36 (PI	ERCAP)		75 M BRIDGE W	/ITH LINEAR LRPB+FVD : NS (M) 1510 KN-M
Case	ELCENTROCOMOBO_NS		d Length Of cation)	fse : 6	
Items		x/Min Env 🗸	LEnd: 0	. m ). m	NS (V) 9363 KN
			Jt	64 .6 m	
	ant Shear			5.2 m)	
Result	ant Snear	1			Shear V2
					1510.288 KN at 3.1 m
					-2063.393 KN at 3.1 m
Resulta	ant Moment				Moment M3
					9363.6838 KN-m
				_	at 0. m -12792.8809 KN-m at 0. m
Re	set to Initial Units	Done			Units KN, m, C 🗸
	a Na Cauffaunatian	Farm			l and /0 /m
2	r No <u>Configuration</u>	Force			Load (0.4g)
		MOMENT (KN-M)	SHEAR(K	N)	ELCENTRO_NS COMPONENT.
	A Description	1504		700	ADDITIONAL DAMPING HAS
	a) Linear/Rigid link	4501		783	INCREASED THE STIFFNESS
1	b) LRPB	1003		2424	THERE BY CONTIBUTING TO
,		•			INCREASED MOMENT AND
	c) LRPB + FVD	1510		9363	SHEAR VALUES.
	CSIBRIDGE INTERFA	UE FOR DECREAS	ED MOM	ENT OF BR	IDGE PIEK

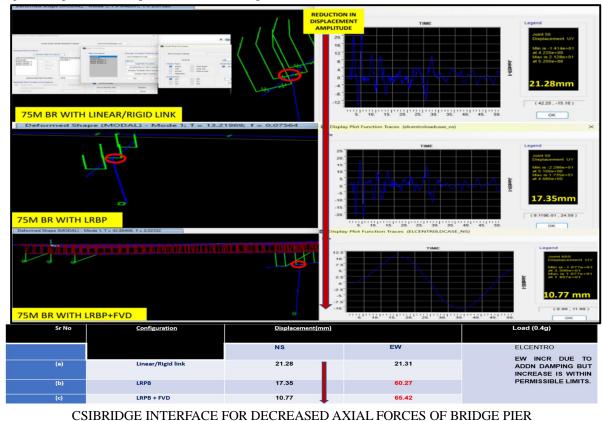


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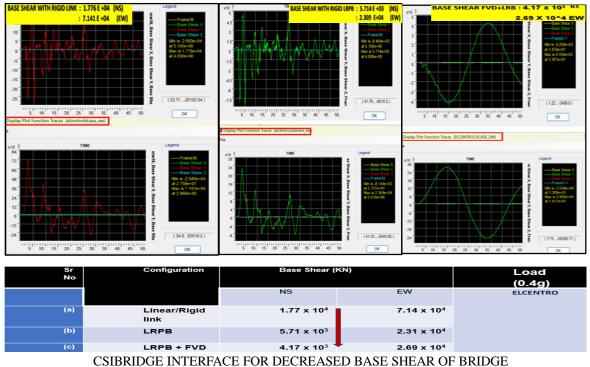
CSIBRIDGE INTERFACE FOR DECREASED AXIAL FORCES OF BRIDGE PIER

4) *Pier Top Response (Uy).* LRBs in bridge piers limit pier top drift by providing flexibility and lateral displacement. They absorb and dissipate energy, reducing displacement during seismic events. Combined with FVDs, they control pier top drift, enhance damping, and improve seismic performance. NS component shows better results.

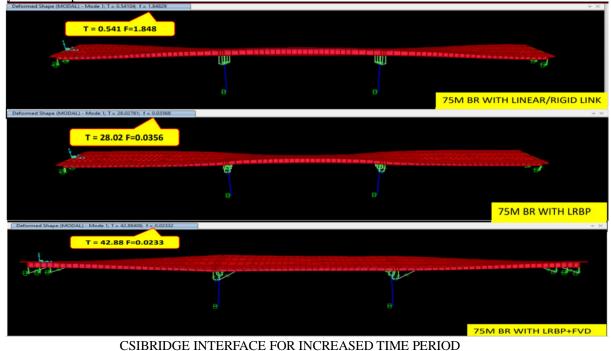




5) Base Shear of System (Transverse Direction). When both Laminated Rubber Bearings (LRB) and Fluid Viscous Dampers (FVD) are used in bridge piers, they can further contribute to reducing the base shear. LRBs provide flexibility to absorb seismic forces, while FVDs add damping properties to dissipate energy. This combined effect helps mitigate the base shear by absorbing and dissipating seismic energy, resulting in improved seismic performance and reduced demands on the bridge pier.



6) *Time And Frequency Response*. The installation of isolators in a bridge reduces its natural frequency, making it less responsive to higher frequency vibrations. This can improve user comfort, reduce fatigue damage, and enhance the bridge's resistance to earthquake loads. The study analysed different bearing configurations and their effects on the bridge's response over time and frequency. The time period as shown below has increased.





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#### IX. CONCLUSIONS

This study examined the effectiveness of VD-LRBPs (Viscous Dampers combined with Bearing) in controlling large displacements between the superstructure and substructure during Elcentro earthquakes in the transverse direction. Detailed FEM modelling, vehicle simulation, and dynamic analysis were conducted. The results showed that the combination of rubber bearings and viscous dampers increased flexibility and reduced the bridge's response to seismic excitation. The addition of viscous dampers also reduced axial forces in most cases but increased shear values, which could be mitigated by additional damping from the bearing and FVD. The exponential damper with  $\alpha$ =1 was found to effectively control displacements, providing a simple methodology for selecting optimal damper characteristics. Transverse moments in the column were reduced, particularly in the NS component. The shift in fundamental time period increased the bridge's flexibility, and the base shear of the bridge structure was significantly reduced after isolation.

#### X. ACKNOWLEDGEMENT

The authors thank Dr DI Narkhede and the Head of the Department of Civil Engineering for providing the necessary facilities and guidance to complete the work successfully.

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