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Experimental Investigation on Repair and Maintenance of Flexible Pavement using Geosynthetics

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Abstract: The activity of vehicles on the surface of street yield distortions in the asphalt structure, these disfigurements corrupt the asphalts and abnormalities, trenches, splits and potholes. Potholes headed for be found in any urban and provincial territory of the nation. In India where the improvement of potholes on the streets and extremely common.as request to keep up these streets, it is important to distinguish the reason for potholes and successful cures. In the greater part of Indian urban communities potholes are repaired with outdated methods, nowdays geosynthetics are ordinarily utilized as a part of adaptable street base strengthened by embeddings them commonly at the interface between the base course and the subgrade. Keywords: Potholes, cause of potholes, repair techniques, geosynthetics, flexible pavement, cyclic plate load.

I. INTRODUCTION

Streets in India are fundamentally bitumen-based macadamized streets. The principle streets in India are under immense weight and in extraordinary need of modernization so as to deal with the expanded necessities of the Indian economy. Notwithstanding support, the extension of the system and broadening of existing streets is winding up progressively imperative. This would then empower the streets to deal with expanded activity, and furthermore take into consideration a comparing increment in the normal development speed on India's streets. Advancement of potholes on Indian streets and avenues after the beginning of rainstorm is a typical wonder. Consistently there is an open objection and daily papers are brimming with pictures demonstrating potholed street asphalts. Hot blend black-top plants are generally closed down amid storms and no hot bituminous blend is accessible for filling potholes. Thusly, numerous potholes are either not repaired or repaired with out of date procedures. Geosynthetics have been effectively used to satisfy various capacities that contribute altogether to the great execution of roadways. They incorporate the elements of partition, stiffening, reinforcement, drainage, separation, and protection. One or on the other hand a greater amount of these different capacities has been utilized as a part of no less than six imperative roadway applications. The applications incorporate the movement of intelligent splitting in black-top overlays, detachment, adjustment of street bases, adjustment of street delicate subgrades, what's more, parallel seepage. This paper outlines the systems and additionally enter propels in every last one of these numerous applications.

II. MATERIALS

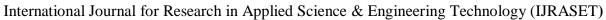
A. Geotextile

The geotextile texture should be a woven, non-woven or weaved texture comprising of long-chain polymeric fibers or yarns, for example, polypropylene, polyethylene or polyester or any blend thereof, framed into a steady system to such an extent that the fibers or yarns hold their relative position to each other. There are a few application territories for geotextiles requiring particular capacities specifically separation, filtration, drainage, reinforcement.

HTSF W-3020A is woven geo-textile made of 100% polypropylene Multifilament yarn, which are made from very high tenacity yarn and very good stability.



Figure 1 Geotextile (HTSF-W3020A)





The table below provides the Physical properties of used geotextile.

Table 1 Physical Properties of Geotextile

Sr.	Design-parameter	HTSF-W3020A
No.		
1	Material Geo-bag textile	100% P.P
2	E.O.S. (mm) Pore size (mm)	>0.15 < 0.70
3	Specific gravity	0.91
4	Thickness in mm (min) at 1KPa Pressure	0.8
5	Breaking Strength from (5cm x 20 m strip) (IS: 1969 – Latest edition)	> 100 K.N.
	Warp	
6	% in Elongation at break (IS: - Latest edition) Warp	<28
7	Grab strength test (3" x1" strip) (ASTM-D- 1682) Warp way (Kgs)	>0.220
8	% in Elongation (Grab test) (ASTM-D- 1682) Warp way	<28
9	Tear Strength (Single rip) (ASTM-D-1682) Wrap way (Kgs)	>0.60
10	Water permeability (Lim/m2/Sec. At 10 cm water head)	>20
11	G.S.M.	430 GSM
12	Width of Fabric .cms	>150
	Length of roll mtrs.	>100

III.OBJECTIVE

- A. To evaluate the comparative performance of pavements with and without geosynthetic stabilization by experimental investigations on subgrade sample.
- B. To provide solution for flexible pavement failure (for potholes) with geosynthetics (geotextile).

IV.EXPERIMENTAL WORK

A. Aggregate testing

The aggregate performance tests were performed on the various specified properties, and the resulting test results were compared with the allowable values in the MORTH specification, as shown in the table.

Table 2 Test Result of 6mm Grit

	TEST RESULT								
	Specification: Morth 5 th Revision								
Sr. No	SIEVE SIZE	% PASSING	REQ.	FLAKINESS INDEX, %	ELONGATION INDEX, %		METHOD OF TEST		
01	9.5 mm	100	100						
02	6.3 mm	93.46	90-100	-	-	-	IS:2386		
03	3.35 mm	51.95	45-65	Combined Shall be < 35% (As per Morth)		(Part-1):1963 Reaffirmed 2016			
04	1.18 mm	12.42	10-30						
05	0.075 mm	3.99	2-8						
				OTHER TEST RESULT	S				
Sr. No	TYPE (OF TEST	RESULT	REQUIREMENT MI		ETHOD OF TEST			
1	Water Ab	sorption, %	0.93	Max 2 %		IS:2386			
2	Specifi	c Gravity	2.829	-		(Part-3)	:1963 Reaffirmed 2016		



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Table 3 Test Result of 10mm Aggregate

	TEST RESULT								
	Specification: Morth 5 th Revision								
Sr. No	SIEVE SIZE	% PASSING	REQ.	FLAKINESS INDEX, %	ELONGATION INDEX, %		METHOD OF TEST		
01	12.5 mm	100	100	17.05	22	22.21			
02	10.0 mm	88.50	85-100	17.03	22.	.21	IS:2386 (Part-1):1963		
03	4.75 mm	5.95	0-20	Combined Shall be < 40 (As per Morth)		0%	Reaffirmed 2016		
04	2.36 mm	0.94	0-5						
			OTH	HER TEST RESULTS					
Sr. No	TYPE (OF TEST	RESULT	REQUIREMENT METHOD OF		THOD OF TEST			
1	Water Abso	orption, %	0.89	Max 2 % IS:2386		IS:2386			
2	Specific Gra	avity	2.868	-		- (Part-3):1963 Reaffin		1963 Reaffirmed 2016	
4	Impact Valu	ue, %	13.95	<45% for Concrete & <30 % for Wearing surfaces				IS:2386	
6	6 Abrasion Value, % 17.84		17.84	<45% for concrete & <30% for wearing surfaces		& (Part-4):1963 Reaffirmed 2010			

Table 4 Test Result of 20 mm Aggregate

	TEST RESULT								
	Specification: Morth 5 th Revision								
	ı	I	Specific	ation: Morting Revisi	OII		1		
Sr. No	SIEVE SIZE	% PASSING	REQ.	FLAKINESS INDEX, %	ELONGATION INDEX, %		METHOD OF TEST		
01	40.0 mm	100	100	15.26	20.14		10.2206		
02	20.0 mm	91.52	85-100	13.20			IS:2386 (Part-1):1963 Reaffirmed 2016		
03	10.0 mm	2.15	0-20	Combined Shall be < 40% (As per Morth)		%			
04	4.75 mm	0.08	0-5						
			OTH	HER TEST RESULTS					
Sr. No	TYPE	OF TEST	RESULT	REQUIREMENT		ME	METHOD OF TEST		
1	Water Abso	orption, %	0.87	Max 2 %		IS:2386			
2	2 Specific Gravity 2.870		-		(Part-3):1963 Reaffirmed 2016				
4	Impact Valu	ıe. %	13.26	<45% for Concrete &					
'	Impact van		13.20	<30 % for Wearing surfaces			IS:2386		
6	6 Abrasion Value, %		16.24	<45% for concrete &		, ,			
Abrasion value, 70		10.2	<30% for wearing surfaces						



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Table 5 Test Result of 40mm Aggregate

	TEST RESULT								
	Specification: Morth 5 th Revision								
Sr. No	SIEVE SIZE	% PASSING	REQ.	FLAKINESS INDEX, %	ELONGATION INDEX, %		METHOD OF TEST		
01	63.0 mm	100	100	11.25 13.27		15.2296			
02	40.0 mm	89.63	85-100	11.23	13	.21	IS:2386 (Part-1):1963		
03	20.0 mm	5.21	0-20	Combined Shall be < 40% (As per Morth)		%	Reaffirmed 2016		
04	10.0 mm	0.52	0-5						
			OTH	ER TEST RESULTS					
Sr. No	TYPE	OF TEST	RESULT	REQUIREMENT		METHOD OF TEST			
1	Water Abso	orption, %	0.81	Max 2 %		IS:2386			
2	Specific Gr	avity	2.873	-		- (Part-3):1963 Reaffirmed			
4	Impact Valu	ıe, %	11.24	<45% for Concrete & <30 % for Wearing surfaces					IS:2386
6	Abrasion V	alue, %	14.36	<45% for concrete & <30% for wearing surfaces		(Part-4):1963 Reaffirmed 20			

Table 6 Test Result of 45-63mm Aggregate

TEST RESULT									
	Specification: Morth 5 th Revision								
Sr. No	SIEVE SIZE	% PASSING	REQ.	FLAKINESS INDEX, %			METHOD OF TEST		
01	90.0 mm	100	100	9.63 11.52					
02	63.0 mm	96.36	90-100	9.03	11	.52	IS:2386		
03	53.0 mm	42.15	25-75				(Part-1):1963 Reaffirmed 2016		
04	45.0 mm	2.85	0-15	Combined Shall be < 35% (As per Morth)		5%			
05	22.4 mm	0.74	0-5						
	·		OTHE	R TEST RESULTS					
Sr. No	TYPE	OF TEST	RESULT	REQUIREMENT ME		THOD OF TEST			
1	Water Abso	orption, %	0.79	Max 2 %		IS:2386			
2	Specific Gravity		2.882	-		(Part-3):1963 Reaffirmed 2016			
4	Impact Val	ue, %	9.85	<45% for Concrete & <30 % for Wearing surfaces		,		IS:2386	
6	Abrasion V	alue, %	13.26	<45% for concrete & <30% for wearing surfaces		(Part-4):1963 Reaffirmed 20			

B. bitumen testing

Bitumen grade was used in this study is VG-30. Sample of bitumen were tested for penetration test, ductility test, viscosity test and softening point test. The test results of different bitumen tests results are shown in table:

Table 7 Bitumen Test Result

	TEST RESULT							
	Method of Test: IS:1201-1220:1978 Reaffirmed 2009							
Sr.	TESTS	RESULTS	REQUIREMENT AS					
No.	12313	RESULTS	PER IS:73-2013					
1	Penetration at 25° C,0.1mm, 100gm, 5s	52	Min 45					
2	Softening Point, ° C	49.5	Min 47° C					
3	Absolute Viscosity at 60° C, Poise	2932	2400-3600					
4	Kinematic Viscosity at 135° C, cSt	429	Min 350					
5	Ductility @ 25° C	95	Min 40 cm					

C. Design and Method

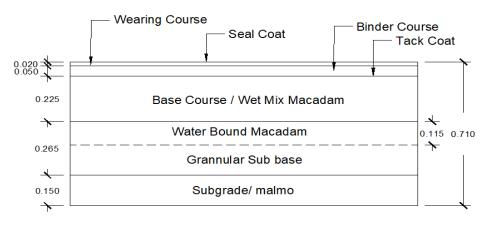
The experimental work is performed on mild steel plate box having following properties:

- 1) Size of Box: 0.8 x 0.8 x 0.8 m.
- 2) Thickness of the plate: 1.20 mm.



Figure 2 Mild Steel Box

- 3) The Cyclic plate load test is to be performed with geotextile as reinforcement, same as with geogrid. (IS: 5249-1992, clause 6)
- 4) The below figure shows the different pavement layers with their dimensions.



*All Dimension are in Meter

Figure 3 Pavement Layers in Prepared Sample



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Pavement Layer Criteria: for the 2 msa traffic with CBR 4%, (Source: IRC:37 (2001) Page No. 23, Pavement Design Catalogue)

- a) Granular Sub base = 265 mm
- b) Base Course = 225 mm
- c) Binder Course = 50 mm
- d) Wearing Course = 20 mm

By the formula of density, we can find the total mass at desirable compaction density.

Density = Mass/volume

Table 8 Layer Composition

Layer	Density (gm/cm³) (As per Morth 5 th Revision)	Volume (m ³)	Total Weight (kg)
Subgrade	1.785	0.096	171
Granular Subbase	1.803	0.096	173
Water Bound Macadam	1.803	0.0736	132
Base Course /Wet Mix Macadam	2.214	0.144	318
Binder Course	2.214	0.032	70
Wearing Course	2.214	0.0128	28

D. Preparation of Pavement Layers



Figure 4 Laying and Compaction of Subgrade



Figure 5 Laying and Compaction of Subbase



Figure 6 Laying and Compaction of WBM



Figure 7 Laying and Compaction of WBM final layer



Figure 8 Laying and Compaction of Base course/WMM



Figure 9 Laying and Compaction of Binder Course



Figure 10 After Completion of Laying and Compaction of Wearing Course





- E. Cyclic Plate Load Test for Normal Sample
- 1) Load Test calculations:
- a) Assume SBC = 40 t/m2
- b) Test load = $40 \times 2.50 = 100 \text{ t/m}2$

Pressure Gauge Least Count = 10 kg/cm2

Ram Dia = 9.62 cm2

 $= 9.62 \times 10$

= 96.2 kg

Plate Area = $0.075 \times 0.075 = 0.005625 \text{ m}$ 2

Total Apply Load = 100×0.005625

= 0.5625 ton = 562.5 kg

Increment nos. = 562.5/5 = 112.5 kg but Actual apply load = 96.2, therefore increment = 6 with maximum load of 96.2 kg.

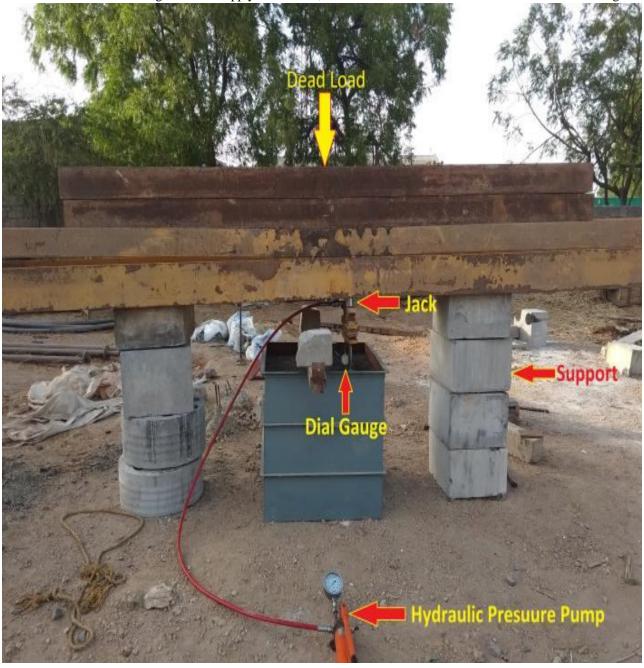


Figure 11 Test Setup



Table 9 Test Result of Cyclic Plate Load for Normal Sample

-		D:-1 C	G.441
Sr. No.	Load (Kg)	Dial Gauge Readings, mm	Settlement in
	Point -	_	mm
0		_	0.00
0	0.0	30.00	0.00
1	96.2	28.10	1.90
Release Reading	0.00	29.05	
2	192.4	27.90	2.10
Release Reading	0.0	28.40	
3	288.6	26.60	3.40
Release Reading		27.22	
2			
4	384.8	26.12	3.88
Release Reading	0.0	26.70	
2			
5	481	25.90	4.10
Release Reading	0.0	2625	
6	562.5	25.38	4.62
Release Reading	0.0	25.90	
<u> </u>	Point -	2	<u> </u>
0	0.0	30.00	0.00
1	96.2	26.78	3.22
Release Reading	0.00	27.21	
2			
2	192.4	23.20	6.80
Release Reading	0.0	24.10	
3	288.6	21.40	8.60
Release Reading	0.0	23.62	
8			
4	384.8	20.22	9.78
Release Reading	0.0	21.58	
		21.50	
5	481	19.73	10.27
Release Reading	0.0	20.95	
6	562.5	19.45	10.55
Release Reading	0.0	20.61	





C. N.	I 1 (IV -)	Dial Gauge	Settlement in
Sr. No.	Load (Kg)	Readings, mm	mm
	Point -	3	
0	0.0	30.00	0.00
1	96.2	27.35	2.65
Release Reading	0.00	27.88	
2	192.4	26.70	3.30
Release Reading	0.0	27.48	
2	200.6	27.10	4.02
Belease Beeding	288.6	25.18	4.82
Release Reading		26.50	
4	384.8	24.65	5.35
Release Reading	0.0	25.60	3.33
Release Reading	0.0	23.00	
5	481	24.08	5.92
Release Reading	0.0	25.32	
8			
6	562.5	23.90	6.10
Release Reading	0.0	24.82	
<u>-</u> 1	Point -	4	1
0	0.0	30.00	0.00
1	96.2	27.90	2.10
Release Reading	0.00	28.12	
2	192.4	26.23	3.77
Release Reading	0.0	27.05	
	200.5	0.7.7.	,
3	288.6	25.55	4.45
Release Reading	0.0	26.60	
4	201 0	24.02	5.00
Release Reading	384.8 0.0	24.02 25.80	5.98
Release Reading	0.0	23.80	
5	481	23.82	6.18
Release Reading	0.0	25.05	0.10
Titles Teading	0.0	25.05	
6	562.5	23.08	6.92
Release Reading	0.0	24.75	
6			

Sr. No.	Load (Kg)	Dial Gauge	Settlement in
51. 110.	Load (Rg)	Readings, mm	mm
	Point -	5	
0	0.0	30.00	0.00
1	96.2	26.88	3.12
Release Reading	0.00	27.60	
2	192.4	25.40	4.60
Release Reading	0.0	26.75	
3	288.6	24.62	5.38
Release Reading	0.0	25.05	
4	384.8	23.72	6.28
Release Reading	0.0	24.62	
5	481	23.39	6.61
Release Reading	0.0	24.15	
6	562.5	22.85	7.15
Release Reading	0.0	23.98	

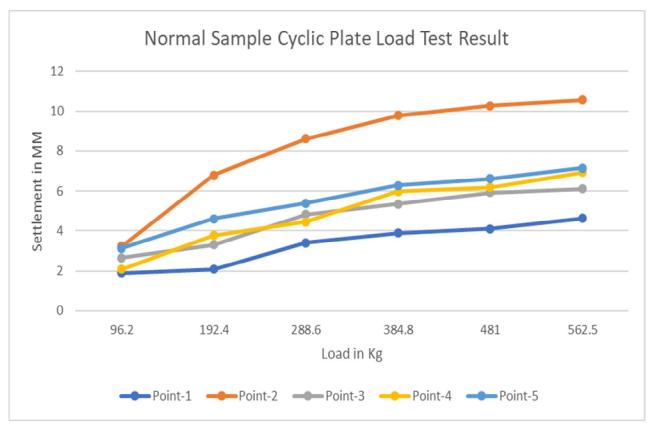


Figure 12 Graphical Representation of Normal Sample Result

F. Cyclic Plate Load Test for Geotextile Based Sample



Figure 13 Position of Pothole

The Fig. 13 Showing the position of the pothole, which is shown in irregular shape. The while square boundary is the marking for the cutting and dig out up to the Base course.



Figure 14 Geotextile Laying

The Fig. 14 Shows the dig out portion and placing of geotextile in the position. The geotextile is nailed at four corners for kept it appropriate position.



Figure 15 Laying of Bitumen Concrete

Fig. 15 showing the laying of bitumen concrete on the geotextile. After fill out the portion, proper compaction is done. On the surface of bitumen concrete the aggregate dust is sprinkled.



Figure 16 Testing on Geotextile Based Sample



Table 10 Test Result of Cyclic Plate Load for Geotextile Based Sample

C. N.	I 1 (IZ-)	Dial Gauge	Settlement in	
Sr. No.	Load (Kg)	Readings, mm	mm	
	Point - 1			
0	0.0	30.00	0.00	
1	96.2	29.80	0.20	
Release Reading	0.00	29.84		
2	192.4	29.44	0.56	
Release Reading	0.0	29.68		
3	288.6	28.92	1.08	
Release Reading	0.0	29.10		
4	384.8	28.55	1.45	
Release Reading	0.0	28.95		
5	481	27.60	2.40	
Release Reading	0.0	28.10		
6	562.5	27.05	2.25	
Release Reading	0.0	27.75		
	Point - 2			
0	0.0	30.00	0.00	
1	96.2	28.87	1.13	
Release Reading	0.00	29.22		
2	192.4	28.33	1.67	
Release Reading	0.0	28.90		
3	288.6	27.69	2.31	
Release Reading	0.0	28.32		
4	384.8	27.24	2.60	
Release Reading	0.0	27.72		
5	481	26.70	3.30	
Release Reading	0.0	27.08		
6	562.5	26.22	3.78	
Release Reading	0.0	26.88		
			1	





Sr. No.	Load (Kg)	Dial Gauge Readings, mm	Settlement in mm
	Point - 3	_	
0	0.0	30.00	0.00
1	96.2	27.78	2.22
Release Reading	0.00	28.44	
2	192.4	27.35	2.65
Release Reading	0.0	27.95	
3	288.6	27.02	2.98
Release Reading		27.60	
4	384.8	26.66	3.34
Release Reading	0.0	27.00	
5	481	26.20	3.80
Release Reading	0.0	26.45	
6	562.5	25.92	4.08
Release Reading	0.0	26.22	
	Point - 4	ļ	
0	0.0	30.00	0.00
1	96.2	28.10	1.9
Release Reading	0.00	28.44	
2	192.4	27.54	2.46
Release Reading	0.0	27.98	
3	288.6	27.33	2.67
Release Reading	0.0	27.92	
4	384.8	26.89	3.11
Release Reading	0.0	27.56	
5	481	26.66	3.34
Release Reading	0.0	27.25	
	# co =	2501	2.0.5
6	562.5	26.04	3.96
Release Reading	0.0	26.97	

Sr. No.	Load (Kg)	Dial Gauge	Settlement in		
		Readings, mm	mm		
Point - 5					
0	0.0	30.00	0.00		
1	96.2	27.65	2.35		
Release Reading	0.00	28.00			
2	192.4	27.17	2.83		
Release Reading	0.0	27.68			
3	288.6	26.59	3.41		
Release Reading	0.0	27.43			
4	384.8	26.13	3.87		
Release Reading	0.0	27.09			
5	481	25.74	4.26		
Release Reading	0.0	26.86			
6	562.5	25.22	4.78		
Release Reading	0.0	26.30			

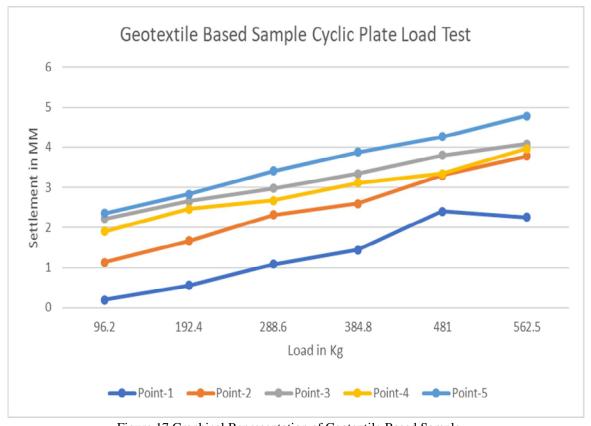


Figure 17 Graphical Representation of Geotextile Based Sample



Table 11 Comparison of Results

Point	Loading	Normal Sample Result,	Geotextile Based Sample	Decrement in
		(Settlement in mm)	Result, (Settlement in mm)	Settlement (%)
192 288 384	96.2	1.90	0.20	89.47
	192.4	2.10	0.56	73.33
	288.6	3.40	1.08	68.23
	384.8	3.88	1.45	62.62
	481	4.10	2.40	41.46
	562.5	4.62	2.25	51.29
2	96.2	3.22	1.13	64.90
19 28 38 4	192.4	6.80	1.67	75.44
	288.6	8.60	2.31	73.13
	384.8	9.78	2.60	73.41
	481	10.27	3.30	67.86
	562.5	10.55	3.78	64.17
3	96.2	2.65	2.22	16.22
_	192.4	3.30	2.65	19.69
	288.6	4.82	2.98	38.17
	384.8	5.35	3.34	37.57
	481	5.92	3.80	35.81
	562.5	6.10	4.08	33.11
4	96.2	2.10	1.9	9.52
	192.4	3.77	2.46	34.74
	288.6	4.45	2.67	40.00
	384.8	5.98	3.11	48.00
	481	6.18	3.34	45.95
	562.5	6.92	3.96	42.77
5	96.2	3.12	2.35	24.67
	192.4	4.60	2.83	38.47
	288.6	5.38	3.41	36.61
	384.8	6.28	3.87	38.37
	481	6.61	4.26	35.55
	562.5	7.15	4.78	33.14
				Avg - 47.12

V. CONCLUSION

Based on the laboratory experiments and analysis, the following conclusions are drawn,

- A. It is observed that, for geotextile-based sample results are decreased than normal sample (Table 11) which indicates that the use of geotextile in repair of potholes can increase the load carrying capacity and gives better strength in repair.
- B. The average decrement in geotextile based sample is 47.12% compared to normal sample.

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