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Effect of Granite Dust & M-Sand on Strength Properties of Conventional Concrete

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Abstract: Concrete Industry is one of the most growing industry in all over world. It mainly consists of Cement, which gives the major impact on environment by producing CO₂, during cement production. The main aim of this study is to minimize CO₂ emission, by producing an alternative cementitious material which can also reduce energy consumptions. Another, constituent material of concrete is Fine Aggregate. The scarcity of fine aggregate leads to developed its alternative material. As, we know India is one of the country which produces huge amount of dimensional stone and it is found locally very easily and abundantly, we aim to use this stone in place of cement as well as fine aggregate. In this work, we used Granite dust as partial replacement of cement at various replacement percentage ranging from 0%,5%,10%,15% & 20%. Alongside, we used M-Sand with a variation percentage 0%,10%,20%,30%,40%,50% & 100%. To understand the fresh properties of concrete, Slump Cone Test, V-Bee Test & Compaction Factor test was performed. To determine the hardened properties of concrete, Compressive Strength, Density, Water Absorption, Flexure Test & Split Tensile Strength were performed. Test results showed that concrete with high M-Sand content provides better results than controlled concrete and Granite powder can be used up to 10% with suitable M-Sand replacements.

Keywords: Granite Dust, Manufacturing Sand, M-30, Fresh Properties, Hardened Properties Introduction

I. INTRODUCTION

Concrete is the basic ingredient for the construction industry for its low cost other than any material. Concrete industry uses the most natural resources, which is high energy consuming, inversely give impact on environment. Cement industry contributing the most in emission of CO₂. Fine aggregate is another strength increasing component of concrete. The commonly used fine aggregate is natural river sand. The consumption of natural river sand is very high due to the extensive use of concrete and the demand of natural river sand is high in countries owing to growth of the country. The non-availability of sufficient quantity of ordinary river sand for making cement concrete is affecting the growth of construction industry in many parts of the country. Manufactured sand (MS) is an approach to improve natural resource efficiency. India is having abundant productivity of dimensional stones. Namely, Granite & Marble are the most used stones in country. Marble is having lesser strength than granite. In this, study we used, Granite powder and M-Sand, which is produced from Granite Stone. Granite powder obtained during cutting stage. It is a waste product. We aimed to developing two new alternative material, which can be used in residential work also locally available, economical and don't compromise with the strength also. The objective of reduction of cost of construction can be met and it will help to overcome the problem associated with its disposal including the environmental problems of the region.

II. LITERATURE REVIEW

Felix Kala and Partheeban [1] scrutinized the utilization of granite powder as fine aggregate in high performance concrete. In their study, the fine aggregate was replaced with the granite powder. Of all the mixtures considered, concrete with 25% Granite Powder (GP25) was found to be superior to other mixtures. Mechanical properties such as compressive strength, split tensile strength and modulus of elasticity in all the ages at both curing temperature of 32oC and 38oC were higher than that of the reference mix. There was an increase in strength as the days of curing increased.

The result suggested that the proper use of the granite powder could produce high performance concrete. At any rate both granite stone and granite powder in concrete are the best choice, where they are available. Shewaferaw Dinku Belay [2] found that the hardened properties of the concrete mixes with partial replacement of natural sand with manufactured sand achieved higher compressive strength.

Using manufactured sand in partial or full replacement to natural sand does not cause any significant cost variation. He also stated that the use of manufactured sand is more suitable for high strength concrete production. Prakash Rao and Giridhar Kumar [3]



inferred that the concrete cubes with crusher dust developed about 17% higher strength in compression, 7% more split tensile strength and 20% more flexural strength than the concrete cubes and beams with river sand as fine aggregate. Logan Andrew Thomas [4] concluded that at the age of 28 and 56 days, specimen moist cured for 7 days exhibited the highest compressive strength and elastic modulus.

Among the three different curing methods, 1-day heat curing generally resulted in the lowest strength. The continually moist-cured specimens which were never allowed to dry out exhibited modulus of rupture values in some cases twice as great as the values attained from the 7-day moist-cured specimens.

The equation published in ACI 363R-92 provided a good estimate of the modulus of rupture and the elastic modulus regardless of the curing method or the compressive strength. The average Poisson's ratio measured from the test specimens was 0.17 which is within the range generally assumed for normal-strength concrete (0.15 - 0.25).

This finding suggests that it is adequate to use the same Poisson's ratio for HSC as that of normal strength concrete. Felix Kala [5] experimentally investigated the use of locally available granite powder as fine aggregate and partial replacement of cement with admixtures in the production of HPC with 28 days strength to the maximum of 60 MPa. The influence of the water cement ratio and curing days on mechanical properties for the new concrete mixes were premeditated.

The percentage of granite powder added by weight was 0, 25, 50, 75 and 100 % as a replacement of sand used in concrete and cement was replaced with 7.5 % silica fume, 10 % fly ash, 10 % slag and the dosage of superplasticiser added 1 % by weight of cement. The test results show clearly that granite powder as a partial sand replacement has beneficial effects on the mechanical properties of high performance concrete. Of all the six mixtures considered, concrete with 25 % of granite powder (GP25) was found to be superior to other percentages of granite powder concrete as well as conventional concrete and no admixtures concrete for all operating conditions.

Hence the following conclusions are made based on a comparison of GP25 with the control concrete, CC. The mechanical properties like the compressive strength, split tensile strength, modulus of elasticity and flexural strength, particularly for all ages higher than that of the reference mix, CC as mentioned below. There was an increase in strength as the days of curing increased. Compressive strength is 6.12 to 22.14 % greater than that of CC. Split tensile strength is 14.88 to 21.95 % higher than that of CC. Modulus of elasticity is 8.85 to 18.89 % higher than that of CC.

Flexural strength is 12.5 to 22.22 % higher than that of CC. The water absorption was about 8 to 14.2 % less than that of conventional concrete mixture. Prince Arulraj et al.[6] carried out an experimental investigation on use of granite powder as an alternative material for fine aggregate in concrete production.

The percentages of granite powder added by weight to sand by weight were 0, 5, 10, 15, 20 and 25. To improve the workability of concrete 0.5 % Superplasticiser was added. This attempt has been done due to the exorbitant hike in the price of fine aggregate and its limited availability due to the restriction imposed by the government of Tamil Nadu. Fifty four cubes and 36 cylinders were cast. Compressive strength and split tensile strength were found.

The test results indicate that granite as replacement sand with granite powder has a beneficial effect on the mechanical properties such as compressive strength and split tensile strength of concrete. Balamurugan MR [7] examined the possibility of using stone waste as replacement of Pozzolana Portland Cement in the range of 5%, 10%, 30%, 40% and 50% by weight of M 25 grade concrete.

They reported that stone waste of marginal quantity as partial replacement to the cement had beneficial effect on the mechanical properties such as compressive strength values for 7, 14, 28 days were less than the ppc cement.

The literature survey encompasses many studies with partial replacement of natural river sand with M-Sand and partial replacement of cement by granite powder. But , there is no significant research on using both as an alternative material. This research work enlightenment that work.

III. EXPERIMENTAL PROCEDURE

A. Materials

In this study, Ordinary Portland cement was used and replaced partially with granite dust at various percentages which were 0%, 5%,10%,15% & 20% by weight of cement properties are listed in Table-1. It has been observed that 50% of particles had a diameter of 7 μ m and 90% of particle had a diameter lower than 50 μ m and a specific gravity of 2.55. The value of Blaine fineness was 1.50m²/g. The concrete mixture consisted of natural river sand, Coarse aggregate & Manufacturing Sand. The cement to water ratio of 0.40 was maintained in all batches and Admixture dosage were changed to maintain the same workability.

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| Parameters | Parameters Results Requirement a Obtained per IS 269 | | Parameters | Results Obtained | Requirement as per IS 8112 |
|--|---|---|---|--|---|
| Physical Analysis | | | Chemical Composition | | |
| Normal Consistency | Normal Consistency 27% | | Loss on Ignition | 1.8 | Not More than |
| Fineness | 285 m ² / | 225 (Minimum) | (% by mass) | | 5.0% |
| | kg | | | | |
| Setting Time (Minutes) | | | Magnesia (MgO) | 0.9 | Not More than |
| Initial | 125 | 30(Minimum) | (% by mass) | | 6.0% |
| Final | 280 | 600(Maximum) | Sulphuric Anhydride | 2.7 | Not More than |
| Compressive Strength (MPa) | | | (SO3) (% by Mass) | | 3.5% |
| 72 ± 1h (3 Days) | 32.2 | 23.0 | Chloride Content | 0.021 | Not More than |
| | | (Minimum) | (% by Mass) | | 0.1% (for general |
| 168 ± 2h (7 Days) | 41.1 | 33.0 | | | purpose) & not |
| | | (Minimum) | | | more than 0.05% |
| | | | | | for pre-stressed |
| | | | | | structures |
| 672 ± 4h (28 Days) | 56.3 | 43.0 | Insoluble Residue | 0.4 | Not More than 5% |
| | | (Minimum) | (% by mass) | | |
| Soundness | | | | | |
| | | | | | |
| Le-Chatelier Expansion | 1.0 | 10.0 | | | |
| (mm) | | (Maximum) | *Specific Gravit | y of Cement | is = 3.15 |
| _ | 0.08 | | *Specific Gravit | y of Cement | is = 3.15 |
| (mm) | 0.08 | (Maximum) 0.8 (Maximum) | - | | |
| (mm) Auto-Clave Expansion (%) | 0.08 TABLE | (Maximum) 0.8 (Maximum) E –II PERCENTAG | E PASSING VALUES FOR C | COARSE AG | GREGATE |
| (mm) | 0.08 | (Maximum) 0.8 (Maximum) E –II PERCENTAG | E PASSING VALUES FOR C bassing for grading Zone-III | COARSE AG | GREGATE tive % weight |
| (mm) Auto-Clave Expansion (%) | 0.08 TABLE | (Maximum) 0.8 (Maximum) E –II PERCENTAG | E PASSING VALUES FOR C | COARSE AG | GREGATE |
| (mm) Auto-Clave Expansion (%) IS Sieve Size | 0.08 TABLE Cumulative | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p | E PASSING VALUES FOR C bassing for grading Zone-III (IS-383,Table-4) | COARSE AG | GREGATE tive % weight etained |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm | 0.08 TABLE Cumulative | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p | E PASSING VALUES FOR C passing for grading Zone-III (IS-383,Table-4) 100 | COARSE AG Cumulat Ro | GREGATE tive % weight etained |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm 10mm | 0.08 TABLE Cumulative 100 89.5 | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p | E PASSING VALUES FOR C passing for grading Zone-III (IS-383,Table-4) 100 85-100 | COARSE AG Cumulat R | GREGATE tive % weight etained 0 10.43 |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm | 0.08 TABLE Cumulative | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p 7 7 | E PASSING VALUES FOR C passing for grading Zone-III (IS-383,Table-4) 100 | COARSE AG Cumulat R | GREGATE tive % weight etained |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm 10mm 4.75mm 2.36mm | 0.08 TABLE Cumulative 100 89.5 9.7' 4.78 | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p 7 7 8 | E PASSING VALUES FOR C passing for grading Zone-III (IS-383,Table-4) 100 85-100 0-20 0-5 | COARSE AG Cumulat R | GREGATE tive % weight etained 0 10.43 90.23 95.22 |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm 10mm 4.75mm 2.36mm | 0.08 TABLE Cumulative 100 89.5 9.7 4.78 ABLE –III PE | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p 7 7 8 ERCENTAGE PASS | E PASSING VALUES FOR C bassing for grading Zone-III (IS-383,Table-4) 100 85-100 0-20 0-5 SING VALUES FOR FINE AC | COARSE AG Cumulat Ro GGREGATE | GREGATE tive % weight etained 0 10.43 90.23 95.22 |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm 10mm 4.75mm 2.36mm | 0.08 TABLE Cumulative 100 89.5 9.7 4.78 ABLE –III PE | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p 7 7 8 ERCENTAGE PASS | E PASSING VALUES FOR C passing for grading Zone-III (IS-383,Table-4) 100 85-100 0-20 0-5 SING VALUES FOR FINE AC % passing for grading Zone-II | COARSE AG Cumulat Ro GGREGATE | GREGATE tive % weight etained 0 10.43 90.23 95.22 nulative % weight |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm 10mm 4.75mm 2.36mm | 0.08 TABLE Cumulative 100 89.5 9.7 4.78 ABLE –III PE | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p 7 7 8 ERCENTAGE PASS | E PASSING VALUES FOR C bassing for grading Zone-III (IS-383,Table-4) 100 85-100 0-20 0-5 SING VALUES FOR FINE AC | COARSE AG Cumulat Ro GGREGATE | GREGATE tive % weight etained 0 10.43 90.23 95.22 |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm 10mm 4.75mm 2.36mm | 0.08 TABLE Cumulative 100 89.5 9.7 4.78 ABLE –III PE Cumulative | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p 7 7 8 ERCENTAGE PASS | E PASSING VALUES FOR C passing for grading Zone-III (IS-383,Table-4) 100 85-100 0-20 0-5 SING VALUES FOR FINE AC % passing for grading Zone-II | COARSE AG Cumulat Ro GGREGATE | GREGATE tive % weight etained 0 10.43 90.23 95.22 nulative % weight |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm 10mm 4.75mm 2.36mm TA IS Sieve Size | 0.08 TABLE Cumulative 100 89.5 9.7 4.78 ABLE –III PE Cumulative | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p 7 8 ERCENTAGE PASS e % Passed | E PASSING VALUES FOR C bassing for grading Zone-III (IS-383,Table-4) 100 85-100 0-20 0-5 SING VALUES FOR FINE AC % passing for grading Zone-II (IS-383,Table-4) | COARSE AG Cumulat Ro GGREGATE | GREGATE tive % weight etained 0 10.43 90.23 95.22 mulative % weight Retained |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm 10mm 4.75mm 2.36mm IS Sieve Size 4.75mm | 0.08 TABLE Cumulative 100 89.5 9.7 4.78 ABLE –III PE Cumulative 9 9 | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p 7 7 8 ERCENTAGE PASS e % Passed 96 | E PASSING VALUES FOR C bassing for grading Zone-III (IS-383,Table-4) 100 85-100 0-20 0-5 SING VALUES FOR FINE AC % passing for grading Zone-II (IS-383,Table-4) 90-100 | COARSE AG Cumulat Ro GGREGATE | GREGATE tive % weight etained 0 10.43 90.23 95.22 mulative % weight Retained 4 |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm 10mm 4.75mm 2.36mm TA IS Sieve Size 4.75mm 2.36mm | 0.08 TABLE Cumulative 100 89.5 9.7 4.78 ABLE –III PE Cumulative 9 9 8 | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p 7 7 8 ERCENTAGE PASS e % Passed 6 1 | E PASSING VALUES FOR C bassing for grading Zone-III (IS-383,Table-4) 100 85-100 0-20 0-5 SING VALUES FOR FINE AC % passing for grading Zone-II (IS-383,Table-4) 90-100 85-100 | COARSE AG Cumulat Ro GGREGATE | GREGATE tive % weight etained 0 10.43 90.23 95.22 hulative % weight Retained 4 9 |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm 10mm 4.75mm 2.36mm IS Sieve Size 4.75mm 2.36mm 1.18mm | 0.08 TABLE Cumulative 100 89.5 9.7 4.78 ABLE –III PE Cumulative 9 9 8 6 | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p 7 7 8 ERCENTAGE PASS e % Passed 6 9 1 1 | E PASSING VALUES FOR C bassing for grading Zone-III (IS-383,Table-4) 100 85-100 0-20 0-5 SING VALUES FOR FINE AC % passing for grading Zone-II (IS-383,Table-4) 90-100 85-100 75-100 | COARSE AG Cumulat Ro GGREGATE | GREGATE tive % weight etained 0 10.43 90.23 95.22 pulative % weight Retained 4 9 19 |
| (mm) Auto-Clave Expansion (%) IS Sieve Size 12mm 10mm 4.75mm 2.36mm 1.36mm 1.18mm 600micron | 0.08 TABLE Cumulative 100 89.5 9.7 4.78 ABLE –III PE Cumulative 99 98 61 | (Maximum) 0.8 (Maximum) E –II PERCENTAG % Passed % p 7 7 8 ERCENTAGE PASS e % Passed 6 9 6 9 1 1 1 1 1 1 1 1 1 1 1 1 1 | E PASSING VALUES FOR C bassing for grading Zone-III (IS-383,Table-4) 100 85-100 0-20 0-5 SING VALUES FOR FINE AC % passing for grading Zone-II (IS-383,Table-4) 90-100 85-100 75-100 60-79 | COARSE AG Cumulat Ro GGREGATE | GREGATE tive % weight etained 0 10.43 90.23 95.22 95.22 pulative % weight Retained 4 9 19 34 |

TABLE - I CEMENT PROPERTIES



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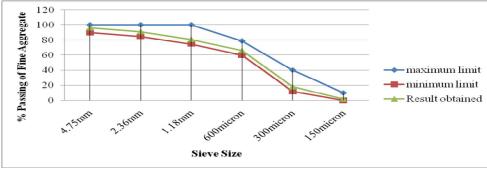


Fig-1 Gradation curve of Fine Aggregate

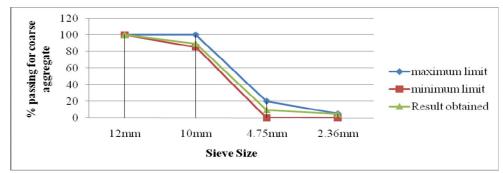


Fig-2 Gradation curve of Coarse Aggregate

| Table IV Percentage Passing Values | s For Manufacture Sand |
|------------------------------------|------------------------|
|------------------------------------|------------------------|

| IS Sieve Size | Cumulative % Passed | % passing for grading Zone-II | Cumulative % weight | |
|---------------|---------------------|-------------------------------|---------------------|--|
| | | (IS-383,Table-4) | Retained | |
| 4.75mm | 93 | 90-100 | 7 | |
| 2.36mm | 89 | 75-100 | 11 | |
| 1.18mm | 68 | 55-90 | 32 | |
| 600micron | 44.9 | 35-59 | 60 | |
| 300micron | 22.6 | 8-30 | 77.4 | |
| 150micron | 9.5 | 0-10 | 90.5 | |

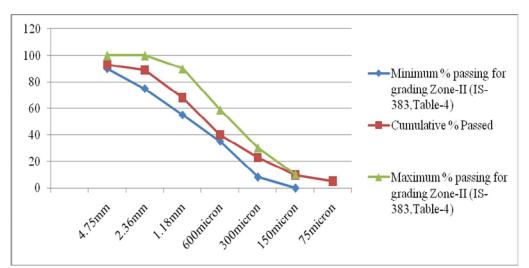


Fig-3 Gradation curve of Manufacturing Sand



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| Bulk density | 1505.0 kg/m3 |
|-----------------------|--------------|
| Close packing density | 1728 kg/m3 |
| Apparent density | 2647.8 kg/m3 |
| Bulk voidage | 43.14% |
| Crushing value | 15.54% |
| Powder content | 4.9% |
| Clay lump content | 0.4% |
| Fineness modulus | 2.69 |

Table V Physical & Chemical Properties Of Manufacturing Sand

B. Mixture Proportions & Test Specimens

Total Five series were casted. In each series, 6 replacements were done. In First series, Granite Powder percentage kept at 0% and M-Sand percentages are varying 10%, 20%, 30%, 40%, 50% & 100%. In Second series Granite Powder percentage increased up to 5% and M- Sand replacement percentages are kept same. In further series, Granite powder increased up to 20% with an interval of 5%. Mix design were done as per IS: 10262 along with IS: 456 guidelines. The compressive strength tests were performed at 7 days & 28 days on 150mm x 150mm x 150mm Cube, the flexural strength test was done on 100mm x 100mm x 500mm Beam at 28 days after curing, in accordance with IS: 516. Split Tensile Strength was done cylinder having diameter 150mm and length 300mm, tested after 28 days, in accordance with IS: 5916. The values are provided in the result table are average of 3 values. The maximum load at failure was recorded for each specimen.

| TABLE VI MIX DESIGN FOR 1 CUM CONCRETE | | | | | | | | | | | |
|--|--------|----------------------|-------------------|--------------------|-------------|----------------------|---------------|-----------|-----------|-----|------|
| Mix No | Cement | Replacement By GP | GP Qty (kg) | Cement Qty (Kg) | Fine Agg | Rep By M- Sand | M-Sand Qty | FA Qty | CA Qty | SP | W/C |
| SCC M0 | 402.5 | 0% | 0 | 402.5 | 703.45 | 0.000 | 0 | 703 | 1183 | 1.0 | 0.4 |
| SCC M1 | 402.5 | 0% | 0 | 402.5 | 703.45 | 10% | 70.345 | 633 | 1183 | 1.0 | 0.4 |
| SCC M2 | 402.5 | 0% | 0 | 402.5 | 703.45 | 20% | 140.69 | 563 | 1183 | 1.0 | 0.4 |
| SCC M3 | 402.5 | 0% | 0 | 402.5 | 703.45 | 30% | 211.035 | 492 | 1183 | 1.0 | 0.4 |
| SCC M4 | 402.5 | 0% | 0 | 402.5 | 703.45 | 40% | 281.38 | 422 | 1183 | 1.0 | 0.4 |
| SCC M5 | 402.5 | 0% | 0 | 402.5 | 703.45 | 50% | 351.725 | 352 | 1183 | 1.0 | 0.4 |
| SCC M6 | 402.5 | 0% | 0 | 402.5 | 703.45 | 100% | 703.45 | 0 | 1183 | 1.0 | 0.4 |
| SCC M110 | 402.5 | 5% | 20.125 | 382.375 | 703.45 | 10% | 70.345 | 633 | 1183 | 1.2 | 0.40 |
| SCC M111 | 402.5 | 5% | 20.125 | 382.375 | 703.45 | 20% | 140.69 | 563 | 1183 | 1.2 | 0.40 |
| SCC M112 | 402.5 | 5% | 20.125 | 382.375 | 703.45 | 30% | 211.035 | 492 | 1183 | 1.2 | 0.40 |
| SCC M113 | 402.5 | 5% | 20.125 | 382.375 | 703.45 | 40% | 281.38 | 422 | 1183 | 1.2 | 0.40 |
| SCC M114 | 402.5 | 5% | 20.125 | 382.375 | 703.45 | 50% | 351.725 | 352 | 1183 | 1.2 | 0.40 |
| SCC M115 | 402.5 | 5% | 20.125 | 382.375 | 703.45 | 100% | 703.45 | 0 | 1183 | 1.2 | 0.40 |
| SCC M210 | 402.5 | 10% | 40.25 | 362.25 | 703.45 | 10% | 70.345 | 633 | 1183 | 1.4 | 0.40 |
| SCC M211 | 402.5 | 10% | 40.25 | 362.25 | 703.45 | 20% | 140.69 | 563 | 1183 | 1.4 | 0.40 |
| SCC M212 | 402.5 | 10% | 40.25 | 362.25 | 703.45 | 30% | 211.035 | 492 | 1183 | 1.4 | 0.40 |
| SCC M213 | 402.5 | 10% | 40.25 | 362.25 | 703.45 | 40% | 281.38 | 422 | 1183 | 1.4 | 0.40 |
| SCC M214 | 402.5 | 10% | 40.25 | 362.25 | 703.45 | 50% | 351.725 | 352 | 1183 | 1.4 | 0.40 |
| SCC M215 | 402.5 | 10% | 40.25 | 362.25 | 703.45 | 100% | 703.45 | 0 | 1183 | 1.4 | 0.40 |

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| TABLE VI MIX DESIGN FOR 1 CUM CONCRETE | | | | | | | | | | | |
|--|--------|----------------------|----------------|--------------------|-------------|----------------------|---------------|-----------|-----------|-----|------|
| Mix No | Cement | Replacement By GP | GP Qty (kg) | Cement Qty (Kg) | Fine Agg | Rep By M- Sand | M-Sand Qty | FA Qty | CA Qty | SP | W/C |
| SCC M310 | 402.5 | 15% | 60.375 | 342.125 | 703.45 | 10% | 70.345 | 633 | 1183 | 1.6 | 0.40 |
| SCC M311 | 402.5 | 15% | 60.375 | 342.125 | 703.45 | 20% | 140.69 | 563 | 1183 | 1.6 | 0.40 |
| SCC M312 | 402.5 | 15% | 60.375 | 342.125 | 703.45 | 30% | 211.035 | 492 | 1183 | 1.6 | 0.40 |
| SCC M313 | 402.5 | 15% | 60.375 | 342.125 | 703.45 | 40% | 281.38 | 422 | 1183 | 1.6 | 0.40 |
| SCC M314 | 402.5 | 15% | 60.375 | 342.125 | 703.45 | 50% | 351.725 | 352 | 1183 | 1.6 | 0.40 |
| SCC M315 | 402.5 | 15% | 60.375 | 342.125 | 703.45 | 100% | 703.45 | 0 | 1183 | 1.6 | 0.40 |
| SCC M410 | 402.5 | 20% | 80.5 | 322 | 703.45 | 10% | 70.345 | 633 | 1183 | 1.8 | 0.40 |
| SCC M411 | 402.5 | 20% | 80.5 | 322 | 703.45 | 20% | 140.69 | 563 | 1183 | 1.8 | 0.40 |
| SCC M412 | 402.5 | 20% | 80.5 | 322 | 703.45 | 30% | 211.035 | 492 | 1183 | 1.8 | 0.40 |
| SCC M413 | 402.5 | 20% | 80.5 | 322 | 703.45 | 40% | 281.38 | 422 | 1183 | 1.8 | 0.40 |
| SCC M414 | 402.5 | 20% | 80.5 | 322 | 703.45 | 50% | 351.725 | 352 | 1183 | 1.8 | 0.40 |
| SCC M415 | 402.5 | 20% | 80.5 | 322 | 703.45 | 100% | 703.45 | 0 | 1183 | 1.8 | 0.40 |

C. Experimental procedures

1) Fresh Properties

Concrete slump test is to determine the workability or consistency of concrete. The slump is carried out as per procedures mentioned in IS: 1199 – 1959. The mould for the test is in the form of the frustum of a cone having height 30 cm, bottom diameter 20 cm and top diameter 10 cm. The tamping rod is of steel 16 mm diameter and 60cm long and rounded at one end. The internal surface of the mould was cleaned and oil applied. Than the mould was placed on a smooth horizontal non- porous base plate. Mould was filled with the prepared concrete mix in 4 approximately equal layers. Each layer has been tempted with 25 strokes of the rounded end of the tamping rod in a uniform manner over the cross section of the mould. For the subsequent layers, the tamping should penetrate into the underlying layer. Excess concrete removed and surface leveled. Raised the mould from the concrete immediately and slowly in vertical direction. Measured the slump as the difference between the height of the mould and that of height point of the specimen being tested.

Compaction factor test is the workability test for concrete conducted in laboratory. The compaction factor is the ratio of weights of partially compacted to fully compacted concrete. Compaction factor apparatus consists of trowels, hand scoop (15.2 cm long), a rod of steel or other suitable material (1.6 cm diameter, 61 cm long rounded at one end) and a balance. Concrete sample has been placed gently in the upper hopper to its brim using the hand scoop and leveled it. After covering the cylinder, trapdoor has been opened at the bottom of the upper hopper so that concrete fall down into the lower hopper. Opened the trapdoor of the lower hopper and allowed the concrete to fall into the cylinder below. Weight the cylinder with concrete to the nearest 10 g. This weight is known as the weight of partially compacted concrete (W1). Empty the cylinder and then refill it with the same concrete mix in layers approximately 5 cm deep, each layer being heavily rammed to obtain full compaction. Level the top surface. Weigh the cylinder with fully compacted. This weight is known as the weight of fully compacted concrete (W2). Find the weight of empty cylinder (W). The Vee-Bee test gives an indication about the mobility and the compactibility aspect of the freshly mixed concrete. The Vee-Bee test apparatus consist of a Vee-Bee consistometer as per IS: 119 - 1959, Initially the sheet metal slump cone is placed inside the cylinder container that is placed in the consistometer. The cone is filled with four layers of concrete. Each concrete layer is one fourth the height of the cone. Each layer after pouring is subjected to twenty-five tamping with the standard tamping rod. The tamping is done with the rounded end of the rod. The strokes are distributed in uniform manner. This must be done in such a way theta the strokes conducted for the second and the subsequent layers of concrete must penetrate the bottom layers. Once the final layer has been placed and compacted, the concrete is struck off to make it in level with the help of a trowel. This makes the cone to be exactly filled. fter the preparation of the concrete cone, the glass disc attached to the swivel arm is moved and is placed on the top of the slump cone placed inside the cylindrical container. The glass disc has to be placed such that it touches the top of the concrete level and the reading is measured from the graduated rod. Now the cylindrical cone is removed immediately by raising the cone slowly in the vertical direction. The transparent disc on the top of the concrete is placed down to the new position and the reading is determined. The difference in the values measured . Now the electrical vibrator is switched on and at the same time we have to start



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the stop watch. The concrete is allowed to spread out in the cylindrical container. Until the concrete is remolded the vibration is continued. This stage is when the surface of the concrete becomes horizontal and the concrete surface completely adheres uniformly to the transparent disc. The time required for complete remolding in seconds is recorded. This time in seconds gives us the measure of workability of the fresh concrete. This time is expressed in Vee-Bee seconds.

D. Hardened Properties

Compressive strength test was done as per IS: 516-1959. Compressive strength of concrete cube test provides an idea about all the characteristics of concrete. This concrete is poured in the mould and tempered properly so as not to have any voids. After 24 hours these moulds are removed and test specimens are put in water for curing. The top surface of these specimen should be made even and smooth. This is done by putting cement paste and spreading smoothly on whole area of specimen. These specimens are tested by compression testing machine after 7 days curing or 28 days curing. Load should be applied gradually at the rate of 140 kg/cm2 per minute till the Specimens fails. Load at the failure divided by area of specimen gives the compressive strength of concrete.

Flexural test was done as per IS: 516-1959.Flexural test evaluates the tensile strength of concrete indirectly. It tests the ability of unreinforced concrete beam or slab to withstand failure in bending. It should be noticed that, the modulus of rupture value obtained by center point load test arrangement is smaller than three-point load test configuration by around 15 percent. Moreover, it is observed that low modulus of rupture is achieved when larger size concrete specimen is considered. Furthermore, modulus of rupture is about 10 to 15 percent of compressive strength of concrete. It is influenced by mixture proportions, size and coarse aggregate volume used for specimen construction.

Tensile strength was done as per IS: 5816-1970. The tensile strength of concrete is one of the basic and important properties. Splitting tensile strength test on concrete cylinder is a method to determine the tensile strength of concrete. The concrete is very weak in tension due to its brittle nature and is not expected to resist the direct tension. The concrete develops cracks when subjected to tensile forces. Thus, it is necessary to determine the tensile strength of concrete to determine the load at which the concrete members may crack.

IV. RESULT & DISCSSION

| | | TABLE VII W | ORKABILITY PRO | PERTIES | |
|----------|------------------|-------------|---------------------|---------------------------|--------------------|
| Mix ID | GP Powder (%) | M-Sand (%) | Slump Value (mm) | Compaction Factor Test | Vee-Bee Test (Sec) |
| SCCM0 | 0% | 0% | <mark>60</mark> | <mark>0.91</mark> | 8.2 |
| SCC M1 | 0% | 10% | <mark>56</mark> | <mark>0.905</mark> | 8.65 |
| SCC M2 | 0% | 20% | 49 | 0.9 | 8.85 |
| SCC M3 | 0% | 30% | 41 | 0.896 | 8.95 |
| SCC M4 | 0% | 40% | 32 | 0.89 | 9.15 |
| SCC M5 | 0% | 50% | 26 | 0.886 | 9.55 |
| SCC M6 | 0% | 100% | 19 | 0.891 | 9.9 |
| SCC M110 | 5% | 10% | 53 | <mark>0.9</mark> | 9 |
| SCC M111 | 5% | 20% | 46 | 0.895 | 9.2 |
| SCC M112 | 5% | 30% | 38 | 0.891 | 9.3 |
| SCC M113 | 5% | 40% | 29 | 0.885 | 9.5 |
| SCC M114 | 5% | 50% | 23 | 0.881 | 9.9 |
| SCC M115 | 5% | 100% | 16 | 0.875 | 9.13 |
| SCC M210 | 10% | 10% | 48 | 0.885 | 10.1 |
| SCC M211 | 10% | 20% | 41 | 0.88 | 10.3 |
| SCC M212 | 10% | 30% | 33 | 0.875 | 10.7 |
| SCC M213 | 10% | 40% | 24 | 0.872 | 11.2 |
| SCC M214 | 10% | 50% | 18 | 0.868 | 11.4 |
| SCC M215 | 10% | 100% | 11 | 0.86 | 11.7 |

A. Fresh Properties



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| SCC M310 | 15% | 10% | 44 | 0.85 | 13.1 |
|----------|-----|------|------|-------|------|
| SCC M311 | 15% | 20% | 36.5 | 0.845 | 13.7 |
| SCC M312 | 15% | 30% | 28 | 0.838 | 13.9 |
| SCC M313 | 15% | 40% | 19 | 0.82 | 14.3 |
| SCC M314 | 15% | 50% | 13 | 0.81 | 15.1 |
| SCC M315 | 15% | 100% | 6 | 0.8 | 16.5 |
| SCC M410 | 20% | 10% | 40 | 0.78 | 19.1 |
| SCC M411 | 20% | 20% | 31 | 0.7 | 19.7 |
| SCC M412 | 20% | 30% | 22 | 0.7 | 21.5 |
| SCC M413 | 20% | 40% | 13 | 0.67 | 22.7 |
| SCC M414 | 20% | 50% | 6 | 0.65 | 23.5 |
| SCC M415 | 20% | 100% | 2 | 0.6 | 25.6 |

Slump Test is important to measure workability of fresh concrete. Very dry mixes having slump 0 - 25 mm are typically used in road making, low workability mixes having slump 10 - 40 mm are typically used for foundations with light reinforcement, medium workability mixes with slump 50 - 90 mm, are typically used for normal reinforced concrete placed with vibration. We have designed to get slump of medium workability with 50mm-90mm. However, for control mix, 10% replacement of M-Sand with 0% GP and 5% GP with 10%M-Sand, we have achieved our desired slump. Other mixes are shown a sign of low workability, which can be used for foundation with light reinforcement.

Compacting factor tests are the most widely used workability tests for concrete. The degree of workability of concrete depends on the values of test results obtained from slump test and compacting factor tests. We have aimed to get a compaction factor value between 0.90-0.935. Which is having medium workability. It shows almost the same pattern shows in slump flow. For, control mix, 10% replacement of M-Sand with 0% GP and 5% GP with 10%M-Sand the CF results are 0.91, 0.905 & 0.90. Which satisfy the requirement of Normal reinforced concrete manually compacted and heavily reinforced sections with vibration. For 0.85-0.87 CF Group, we can use this as Roads, vibrated by hand-operated machines. Concrete may be manually compacted in roads using aggregate of rounded or irregular shape. Mass concrete foundations without vibration or lightly reinforced sections with vibration. For 0.78-0.80 CF, Roads vibrated by power-operated machines. At the more workable end of this group, concrete may be compacted in certain cases with hand-operated machines.

The workability of fresh concrete is a composite property, which includes the diverse requirements of stability, mobility, Compactability, Placeability and Finishability. There are different methods for measuring the workability. Each of them measures only a particular aspect of it and there is really no unique test, which measures workability of concrete in its totality. This test gives an indication of the mobility and to some extent of the Compactibility of freshly mixed concrete. Our results indicate that the concrete is ranging from semi stiff to stiff.

B. Hardened Properties

The compressive strength of concrete, commonly considered to be its most important characteristic, also gave good overall of the quality of the concrete and the structure of the hydrated cement paste. Table shows the calculated average result from 3 specimens at 7 days and 28 days of compressive strength and 28 days of split tensile strength along with 28 days of flexure strength. Figure, illustrates the test results of the compressive strength of concrete with a 0% replacement of Granite powder along with various replacement percentage of M-Sand compared to the control that does not contain GP & M-Sand. As expected, the replacement of the M-Sand affected the compressive strength at 7 days & 28 days. At 28 days, It gives best result 30% replacement and gives less value at 100% replacement. However at 40% replacement level it shows a good result. After, 30% replacement at every 10% replacement, the percentage reduction is 5% and it's continued up to 100%.At, 5% Replacement of cement by GP, with 30% replacement of M-Sand, provides high strength.



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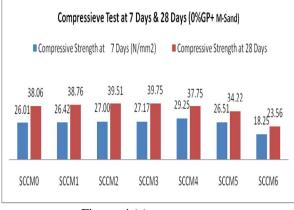
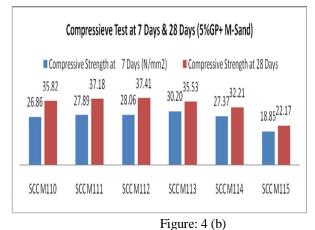


Figure: 4 (a)





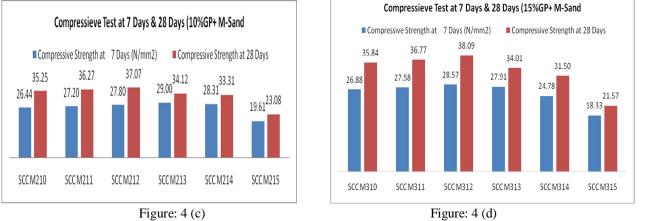
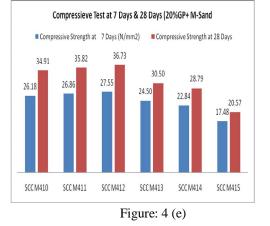
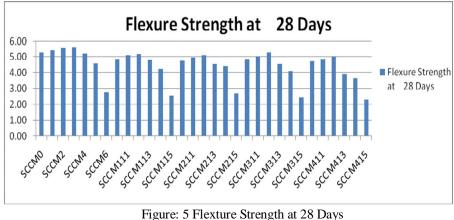


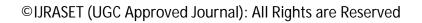
Figure: 4 (a, b. c ,d & e) shows compressive strength of concrete at 7 days & 28 days with various replacement of M-Sand & Granite Powder

At, 10% Replacement of cement by GP, with 30% replacement of M-Sand, provides high strength. At, 15% Replacement of cement by GP, with 30% replacement of M-Sand, provides high strength than 5% & 10% replacements. At, 20% Replacement of cement by GP, with 30% replacement of M-Sand, provides high strength than other replacement levels of M-Sand with GP Powder 20% but it gives lower strength in accordance with the replacement percentage levels of Granite powder.

The Flexural strength of concrete was tested using the two-point loading test method. Flexure test indicated that 30% replacement of M-Sand gives better result at any other replacement. Also, it indicates that 15% replacement of cement with granite dust provides a good result in respect of flexure strength.









Split tensile strength results are also shows the same pattern obtained in flexure strength. It gives 20% more strength increment when we use M-Sand. Also at 15% replacement of cement with GP, increases the strength respectively other percentages. Due to ITZ character it provides more strength.

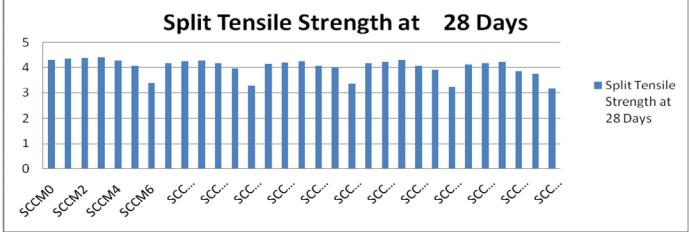


Figure: 6 Tensile Strength at 28 Days

V. CONCLUSION

Granite powder particles are having diameter more than cement particle size and less than fine aggregate, which makes the concrete more denser and increased packing density.

Due to higher packing density it can be predict that water permeability will be less as it is less porous than controlled concrete.

During Flexure & tensile test data analysis, we found there is a strong positive correlation between them. $R^2=0.995$.

Granite powder is acted as good replacement along with 30% replacement of M-Sand. M-Sand gives more strength than controlled concrete at various percentage level as shown in result.

This all are the initial tests, durability test need to be performed to understand long-term behaviour of concrete.

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